# Geotechnical Engineering Report

Sycolin Road Widening Phase IV VDOT Project No. U000-253-312, P101, R201, C501 Leesburg, Virginia

## **Prepared for**

Rinker Design Associates, P.C. August 21, 2015 (Rev. February 4, 2016, April 7, 2016, May 25, 2016)



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Mr. Mark Gunn, P.E. Rinker Design Associates, P.C. 9385 Discovery Boulevard, Suite 200 Manassas, Virginia 20109

Reference: Geotechnical Engineering Report

Sycolin Road Widening Phase IV

VDOT Project No. U000-253-312, P101, R201, C501

Leesburg, Virginia

DMY Project No. 01.02095.01

Dear Mr. Gunn:

**DMY Engineering Consultants Inc. (DMY)** is pleased to submit this report of our geotechnical exploration for the above-referenced project. This report presents our understanding of the project information provided to us, a discussion of the encountered site and subsurface conditions, our field exploration and laboratory testing programs, the field and laboratory test results, and our geotechnical recommendations related to the proposed improvements. The report was revised on April 7, 2016 to address the review comments provided by VDOT on March 21, 2016. The report was revised on May 25, 2016 to reflect the minor design update to the proposed underground detention facility. Please note that no recommendation was changed in the latest revision.

We appreciate the opportunity to be of service to you on this project and would be happy to discuss our findings with you. We look forward to serving as your geotechnical engineer on the remainder of this project and on future projects.

PENG ZHANG Lic. No. 048994

Respectfully,

DMY ENGINEERING CONSULTANTS INC.

Paul Li, PhD, PE

**Project Engineer** 

Peng "Paul" Zhang, PE Principal Engineer

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#### 1.0 PROJECT OVERVIEW

The Sycolin Road Widening Phase IV project is located in Leesburg, Virginia. The project consists of widening approximately 0.9 miles of Sycolin Road from south of Tolbert Lane to Claudia Drive. The site location is show on Figure 1 – Site Location Map in Appendix A.

Based on the project design drawings dated May 2015 prepared by Rinker Design and Associates, P.C. (RDA), the project consists of widening the existing two-lane Sycolin Road into a four-lane divided roadway with dedicated turn lanes at intersections. Other associated improvements include new curbs and gutters, a new shared use path, one culvert extension, and one stormwater detention facility within the project limits. The proposed construction work begins at Station 106+25 and extends generally north to end at Station 151+00.

The description of the proposed project given in the sections above is based on the information provided to us by RDA and information gathered during our site reconnaissance. If any of the assumptions or project information is incorrect, DMY should be informed so that we may revise our geotechnical recommendations, if necessary.

#### 2.0 SITE GEOLOGY AND LOCAL SOIL MAPPING

#### 2.1. REGIONAL GEOLOGY

Based on a review of the Virginia Geologic Map Data available through USGS's online resources (<a href="http://mrdata.usgs.gov/geology/state/state.php?state=VA">http://mrdata.usgs.gov/geology/state/state.php?state=VA</a>), the project site is located within the West Piedmont Physiographic Province of Northern Virginia and is underlain by Newark Supergroup of the Upper Triassic Age.

In the Piedmont province, residual soils have developed from the in-place chemical and physical weathering of the underlying parent bedrock. The soils associated with this geology typically consist of sandy clays, silts, and silty sand materials along with varying amounts of weathered rock fragments and mica. With increasing depth, soil increases in granularity and strength, gradually transitions into highly weathered or Intermediate Geomaterials, and eventually transitions into competent bedrock.

An important aspect of the Piedmont subsurface profile is that highly variable conditions may exist over relatively short horizontal distances. This is caused by variation in mineral composition of the parent rock and the intensity of fractures and joints within the rock. Zones of highly weathered rock may be encountered within residual soils, and lenses of soil may occur in the rock mass. The subsurface profile may be altered by man, by excavating or filling, or by effects of water through the process of erosion or alluvial deposition.

#### 3.0 FIELD EXPLORATION

The field exploration consisted of drilling Standard Penetration Test (SPT) borings, coring the existing pavement, and obtaining bulk samples for laboratory testing. The field exploration procedures are included in Appendix B. Following field operations, the soil samples were transported to our laboratory for further analysis and testing.

A total of 19 pavement cores were obtained along the existing roadway. A total of 37 SPT borings were drilled using either a truck mounted CME-45 C drilling rig or a track mounted CME-55 drilling rig with hollow-stem auger method. In addition to the SPT soil samples, 5 bulk soil samples were collected from the soil borings for laboratory testing. Upon completion of the field exploration, all boreholes were backfilled with compacted auger cuttings. Groundwater levels, where encountered, were measured at each boring location at the time of drilling and 24 hours after drilling, when it was feasible to keep the borehole open for a prolonged period. A solid PVC pipe was inserted in the open boreholes of the SWM and culvert borings to obtain the stabilized groundwater levels. The test locations were selected by DMY and were surveyed in the field by RDA. The locations of the borings and pavement cores are shown on Figure 1 – Boring & Core Location Plan in Appendix A.

#### 4.0 PAVEMENT AND SUBSURFACE CONDITION

#### 4.1. PAVEMENT CONDITION

Pavement cores were obtained at a total of 19 locations. The pavement core information is summarized in Table B1 – Summary of Pavement Core Information in Appendix B. The photographs of the pavement cores are also included in Appendix B.

A total of 16 pavement cores (CO-1 through CO-4, CO-6 through CO-8, CO-10 through CO-12, and CO-14 through CO-19) were obtained on the mainline of Sycolin Road, including turn lanes, acceleration lanes, and striped zones. The pavement cores showed asphalt concrete pavement thicknesses ranging from 7.0 to 12.0 inch with an average of 9.7 inches. The asphalt pavement was underlain by 6.0 to 17.0 inches of aggregate subbase with an average of 12.5 inches.

Three pavement cores (CO-5, CO-9, and CO-13) were obtained from the cross roads. The pavement cores showed asphalt concrete pavement thicknesses ranging from 4.5 to 13 inches. The asphalt pavement was underlain by 10.0 to 13.0 inches of aggregate subbase.

#### 4.2. SUBSURFACE CONDITION

The subsurface conditions encountered at the locations explored are shown in the boring logs in Appendix B. The records represent our interpretation of the subsurface conditions based on visual observations and laboratory testing of the soil samples collected. The lines designating the interfaces between various strata on the boring logs are approximate, as the actual transitions between soil strata are often gradual. In the absence of foreign substances, it is difficult to distinguish between natural soils and clean soil fills. Although individual test borings are representative of the subsurface conditions at the boring locations on the dates shown, they are not necessarily indicative of the subsurface conditions at other locations or at other times.

A layer of topsoil approximately 1 to 5 inches thick was encountered in the majority of the borings drilled in the unpaved areas. Although the measured topsoil depths were found to be of minimal thickness during our exploration, the topsoil may be thicker in unexplored areas, particularly in vegetated undisturbed areas.

Below the topsoil or existing pavement, three basic strata were encountered at the borings within their termination depths. The soil strata included man-made fill, residual soils, and Intermediate Geomaterials (IGM). The borings drilled in the investigation encountered auger refusal at 13 locations. The following table summarizes the generalized soil descriptions from the available subsurface exploration.

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**Table 4-1: Summary of Subsurface Conditions** 

Geologic	Soil Description	USCS Classification		SPT N-	SPT N-values	
Strata	Son Description	Primary	nary Secondary		Max	
FILL	Generally composed of soils obtained from nearby natural soils. The soils from this stratum generally retain some of the properties of the parent soil, but may contain debris and organic material.	ML & SM	SC, CL & GC	6	48	
Piedmont Residual	Natural residual soils derived from the weathering of the inplace natural rock. The soils of these substrata have weathered to the point of no longer retaining any relic rock structure.	ML, CL, SM & SC	MH, CH, GC & SP- SM	6	65	
IGM – Decompos ed Rock  IGM – Decompos of penetration.  IGM is defined as natural residual soils derived from the weathering of the in-place natural rock. IGM is generally considered an intermediate material between soil and rock. VDOT defines IGM as having a minimum SPT N- value of 50 blows per 6 inches of penetration.		GC, ML & SC	SM	50/6"	50/0"	

#### 4.3. GROUNDWATER

Groundwater was measured at the time of drilling and, if feasible, also at 24 hours after drilling completion. Groundwater was only encountered in Boring CL-03 at the depths of 7.5 feet (EL. 350.34 feet) at the time of drilling and 8.0 feet (EL. 349.84 feet) at 24 hours after drilling completion, respectively. It should be noted that groundwater levels fluctuate with seasonal and climatic variations and may be different at other times and locations than those stated in this report.

5.0 LABORATORY TESTING AND RESULTS

### **5.1. LABORATORY TESTING**

Representative soil samples were selected and tested in our laboratory to verify field classifications and to determine pertinent engineering properties. The table below is a summary of the laboratory testing program:

**Table 5-1: Summary of Laboratory Tests** 

Lab Test	Test Method	Test Amount
Natural Moisture Content	AASHTO T-265	116
Atterberg Limits	AASHTO T-89 & T-90	36
Grain Size Analyses	AASHTO T-88	36
Standard Proctor	VTM-1	5
CBR	VTM-8	5

#### 5.2. LABORATORY TEST RESULTS

Natural moisture content of the SPT samples ranged from 2.3 to 38.1 percent. The maximum dry density results ranged from 118.8 to 135.7 pounds per cubic foot (pcf) as obtained using the standard Proctor compaction test method (VTM-1). The corresponding optimum moisture content results ranged from 8.9 to 13.0 percent. The California Bearing Ratio (CBR) values as obtained using VTM-8 ranged from 14.5 to 22.7 with an average of 18.9. The laboratory testing results are included in Appendix C.

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#### 6.0 GEOTECHNICAL RECOMMENDATIONS

#### 6.1. PAVEMENT

We have used the traffic data provided by RDA to perform the pavement design. The provided design traffic data are summarized in the following table.

**Table 6-1: Design Traffic Data** 

	Sycolin Road	Temporary Pavement
Design Life (years)	20	1
Initial Design Year	2019	2019
ADT (2019)	16,236	16,236
ADT (2039)	24,126	16,561
Growth Rate	2	2
Percent Cars/Passenger Vehicles	88.7%	88.7%
Percent Single Unit Trucks and Buses	10.5%	10.5%
Percent Tractor Trailer Trucks	0.8%	0.8%
Total Calculated Design Lane ESALs	3,685,390	151,679

The design CBR value for the pavement design was derived from the 5 CBR tests on the soil samples collected along the proposed roadway areas. The laboratory CBR values ranged from 14.5 to 22.7. We recommend a design CBR value of 6.7 be used for this project.

The pavement design was performed in accordance with the *Guidelines for 1993 AASHTO Pavement Design*, which was published by VDOT and revised in July 2011. Detailed pavement design including the traffic load information and calculations are attached to this report in Appendix D.

We recommend the full depth pavement widening for the mainline using the section in the following table:

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**Table 6-2: Full Depth Pavement Recommendations** 

Roadway Stations	Pavement Section	Remarks
Sycolin Road	Surface Course (SM-9.5D) = 2" Intermediate Course (IM-19.0A) = 2" Base Course (BM-25.0A) = 6" Subbase Course (21B) = 12"	Applies to widening and reconstruction

The pavement construction should be performed in accordance with VDOT standard detail WP-2 for asphalt pavement widening.

VDOT standard UD-4 edge drains should be installed beneath the curb and gutter of all new pavements. VDOT standard UD-2 median drains should be installed in all new raised grass median areas. The underdrain pipes should be either connected to existing underdrain or storm structures.

The existing pavement may be resurfaced by milling and overlay of 2 inches of SM-9.5D with the exception of the airport access road, which only showed 4.5 inches of asphalt pavement in the pavement core. We recommend the airport access road pavement, where it will be used as future Sycolin Road, be reconstructed using the above recommended full depth pavement section.

Temporary pavements will be needed to maintain traffic during construction. A design life of 12 months was used for the temporary pavement. Where new temporary pavement is to be constructed, we recommend that the pavement sections to consist of the following:

**Table 6-3: Temporary Pavement Section** 

Temporary Pavement Section	Surface Course (SM-9.5D) = 1.5" Base Course (BM-25.0A) = 3" Subbase Course (21B) = 6"
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#### 6.2. SITE PREPARATION

Site preparation should consist of any proposed demolition, removing existing underground utilities, existing structures, topsoil and vegetation, and any other soft or unsuitable material from the proposed construction areas. Utilities such as pipes should be removed entirely or abandoned by filling the pipe with grout to prevent future migration of soils into the pipe. Voids resulting from the removal of tree stumps should be filled with compacted structural fill. Disposal of demolition debris should be performed in accordance with local, state and federal regulations. Additional requirements included in Section 301 – Clearing and Grubbing of the VDOT 2007 Road and Bridge Specifications should be followed.

#### 6.3. EARTHWORK

The earthwork for the proposed roadway construction should be performed in accordance with Section 303 – Earthwork of the VDOT 2007 Road and Bridge Specifications. Additional site specific recommendations are discussed in the following paragraphs.

#### 6.3.1. SUBGRADE PREPARATION

Following the site preparation and any required excavation, the newly exposed roadway subgrade should be evaluated by an authorized representative of the Geotechnical Engineer of Record. During this evaluation, we recommend that all subgrade areas be proof-rolled using a fully loaded tandem axle dump truck (10-ton minimum) or similar rubber-tired vehicle. The proofrolling should be performed in such a pattern that the entire subgrade areas are loaded with at least one pass. Areas that are not accessible to proofrolling may be evaluated using other suitable methods such as a steel probe rod.

If the subgrade exhibits excessive deflections or pumping when proof-rolled or soft subgrade is detected by probing, an appropriate remedial measure would be recommended by the Geotechnical Engineer of Record at that time. Potential problem subgrade areas as identified by this soil investigation and the recommended remedial measures are detailed in the following paragraphs. The stabilized subgrade areas should be again evaluated and approved by the Geotechnical Engineer of Record prior to fill placement or pavement installation.

#### 6.3.2. UNSUITABLE SOILS

The recommendations below are for potentially unsuitable soils focused within the upper 5 feet below the proposed pavement. Soils below this depth were generally not considered as they will likely below the zone of influence of the pavement. Unsuitable soils generally include soils that have excessive moisture, high plasticity (Liquid Limits greater than 50%), soils with low SPT N-values (generally less than 5 bpf), low CBR value (less than 6.7), and soils that contains excessive debris or organics. Thicker topsoil and root matter than what was encountered in the borings should be anticipated in wooded areas. The unsuitable pavement subgrade soils at the soil test boring locations are summarized in Table B2 – Summary of Pavement Subgrade Soil Conditions in Appendix B. The types and extent of the unsuitable soils are discussed in detail in the following paragraphs and were estimated based on individual soil test borings and laboratory test results.

#### Highly Plastic or High Swell (>5%) Subgrade Soils:

The soils at the proposed subgrade were found to be highly plastic soils (CH and MH soils) with Liquid Limits greater than 50% in some areas. We recommend that the top 2 feet of the in-situ subgrade soils in these areas be removed and replaced with VDOT Select Material Type I (minimum CBR 30). The removed soils should not be used as roadway embankment fill, but may be used to flatten slopes

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upon approval be the Geotechnical Engineer of Record. The approximate locations where highly plastic soils are encountered at the proposed subgrade are:

Table 6-4: List of Areas with Highly Plastic Soil Subgrade

Roadway Stations	Widening Locations
143+00 to 145+00	Right
146+00 to 148+00	Left

#### Low CBR Subgrade Soils:

Low CBR (<6.7) soils were not encountered at the proposed subgrade elevations in any of the borings.

#### Soft or Very Loose Subgrade Soils:

Soft or very loose soils (SPT N-Values less than 5 bpf) were not encountered at the proposed subgrade elevations in any of the borings. If encountered during construction, the soft/loose subgrade soils should be densified in place.

#### **Excessively Moist Subgrade Soils:**

We have considered that soils with moisture content of greater than 120% of the optimum moisture contents to be excessively moist. In general, excessive moisture may be more or less depending on the amount of recent precipitation at the time construction is performed. Along some parts of the project alignment, the soils at/near the proposed subgrade were found to be excessively moist. This condition has the potential to cause pumping problems during subgrade and base construction. We recommend that upon completion of any necessary excavation in these areas, 2 feet of subgrade soils from beneath the proposed pavement and shoulders be removed, and either dried out and replaced, or replaced with drier soils. Alternatively, the subgrade may be improved in-place using lime or cement treatment. If lime or cement treatment is considered, bulk samples of the material should be obtained of areas requiring treatment to determine the required lime and cement content as well as the method of hydration and compaction required. The approximate locations where excessive moisture contents will likely be encountered are:

Table 6-5: List of Areas with Excessively Moist Subgrade

Roadway Stations	Widening Locations
118+00 to 120+00	Left
124+00 to 128+00	Left
127+00 to 131+00	Right
136+00 to 138+00	Left

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137+00 to 139+00	Right
140+00 to 142+00	Left
141+00 to 143+00	Right

#### 6.3.3. COMPACTED FILL

All engineered fills including roadway embankment and backfill around structures should have a Liquid Limit less than 45 and a Plasticity Index less than 20. Additionally, any borrow material to be used within 3 feet of the pavement subgrade elevation should have a minimum CBR value of 10. Before field operations begin, a representative sample of each proposed engineered fill should be collected and tested to determine its Atterberg Limits, gradation, maximum dry density, optimum moisture content, and natural moisture content. The test results will be used to evaluate the suitability of each proposed engineered fill for quality control purposes during fill placement.

Engineered fill materials should be placed in lifts not exceeding 8 inches in loose thickness for roadway embankments. In confined areas such as utility trenches, portable compaction equipment and thin lifts of 3 to 4 inches will likely be required to achieve the specified degrees of compaction. The engineered fill should be moisture conditioned to within 20 percent of the optimum moisture content and compacted to a minimum of 95 percent of the maximum dry density obtained in accordance with VTM-1, Standard Proctor Method. The top 6 inches of soil supporting pavements, sidewalks, or gutters should be compacted to a minimum of 100 percent of the maximum dry density in accordance with VTM-1, Standard Proctor Method.

#### 6.3.4. CUT AND FILL SLOPES

Most of the proposed construction for the new pavement will take place at or near existing grades. Where new cut and fill slopes are to be constructed, we recommend the slopes be no steeper than 2H: 1V. Soil slopes should be covered for protection from rain. Surface run-off should be diverted away from the slopes. For erosion protection, a cover of grass or other vegetation should be established on permanent soil slopes as soon as possible. Temporary erosion protection of slopes should meet state and/or local requirements.

Where fill materials will be placed to widen existing fills, or placed upon sloping ground, the soil subgrade should be scarified and the new fill benched or keyed into the existing material in accordance with Section 303.04 (h) – Embankments of the VDOT 2007 Road and Bridge Specifications.

#### 6.4. DRAINAGE STRUCTURES

Two drainage structures are planned as part of the proposed construction. The following table is a summary of the drainage structures with a size of 36 inches or greater.

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Table 6-6: Summary of Drainage Pipes and Culvert

Structure I.D.	Approximate Location	Structure Dimensions	Approximate Invert Elevation	Reference Borings
7-9	Sta. 123+25 Left & Right	Double 5'x3' (approximately 90')	358.34' to 359.75'	CL-01 and CL-02
8-9 to 8-11	Sta. 126+25 Right	54" RCP (approximately 50')	350.25' to 350.68'	CL-03

The proposed culvert should be constructed in accordance with Section 302 – Drainage Structures of the VDOT 2007 Road and Bridge Specifications and the standard detail PB-1 of the VDOT 2008 Road and Bridge Standards. Additional foundation recommendations are provided as follows:

Fat CLAY (CH) was encountered in Boring CL-01 at the proposed invert elevation. Highly plastic soils (CH or MH) are unsuitable for direct support of drainage structures. We recommend that the highly plastic soils be undercut a minimum of 24" and replaced with compacted VDOT No. 25 or 26 bedding materials. This condition will likely be encountered on the left side of the roadway along the Double 5'x3' box culvert extension (Structure 7-9).

IGM was encountered in Borings CL-03 at or above the proposed invert elevations of Structures 8-9 to 8-11. Difficult excavation involving a large trackhoe equipped with a hoe ram may be needed to achieve the proposed grades. A minimum 6 inches of bedding materials consisting of compacted VDOT No. 25 or 26 should be maintained between the structure and subgrade. If groundwater was encountered above the proposed foundation subgrade during construction, we recommend that a layer of #57 stone be placed to a minimum of 6 inches above the groundwater prior to placing the regular bedding materials.

#### 6.5. STORMWATER MANAGEMENT FACILITIES

One underground stormwater detention facility is proposed as part of the construction. The detention facility features quadruple 54-inch reinforced concrete pipes (RCP). The following table provides a summary of the facility.

**Table 6-7: Summary of Stormwater Detention Facility** 

Structure I.D.	Approximate Location	Structure Dimensions	Approximate Invert Elevation	Reference Borings
8-22 to 8-23	Sta. 126+65 to 127+90, Right	Quadruple 54" RCP (approximately 125')	355.0' to 355.75'	SWM-03, SWM-04 and CL-03

The proposed underground detention facility should be constructed in accordance with Section 302 – Drainage Structures of the VDOT 2007 Road and Bridge Specifications and the standard detail PB-1 of the VDOT 2008 Road and Bridge Standards.

IGM and auger refusal materials was encountered above the proposed structure invert elevation in Borings SWM-03 and SWM-04. Difficult excavation involving a large trackhoe equipped with a hoe ram may be needed to achieve the proposed grades. A minimum 6 inches of bedding materials consisting of compacted VDOT No. 25 or 26 should be maintained between the structure and subgrade. If groundwater was encountered above the proposed foundation subgrade during construction, we recommend that a layer of #57 stone be placed to a minimum of 6 inches above the groundwater prior to placing the regular bedding materials.

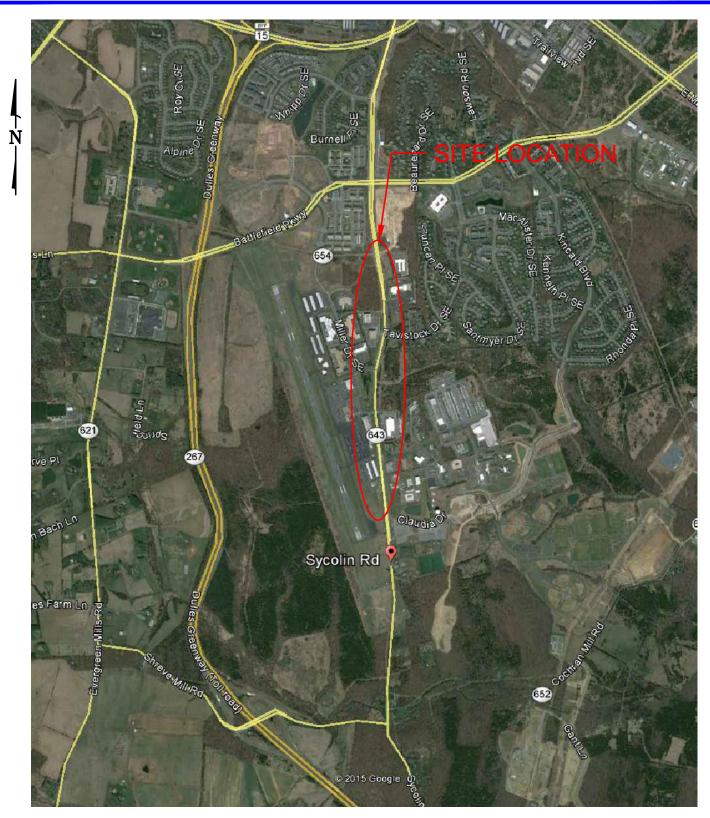
#### 7.0 LIMITATIONS

The recommendations provided are based in part on project information provided to us and are only applied to the specific project and site discussed in this report. If the project information section in this report contains incorrect information or if additional information is available, DMY should be contacted to review our recommendations. We can then modify our recommendations for the proposed project.

Regardless of the thoroughness of a subsurface investigation, there is always a possibility that subsurface conditions may vary from those documented during a subsurface exploration at specific locations. In addition, the construction process itself may alter subsurface conditions. Therefore, experienced geotechnical personnel should be engaged to observe and document the construction procedures used and the conditions encountered. Unanticipated conditions and inadequate procedures should be reported to the design team along with timely recommendations. We recommend that DMY be retained to provide this service based upon our familiarity with the project, the subsurface conditions, and the intent of the recommendations.

We have prepared this report for use by the design professionals for design purposes in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made as to the professional advice included in this report.





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SITE LOCATION MAP



DMY ENGINEERING CONSULTANTS INC. 45662 TERMINAL DRIVE, SUITE 110 DULLES, VIRGINIA 20166 PHONE: (703) 665-0586 FAX: (202) 688-1918

# SYCOLIN ROAD WIDENING PHASE IV LEESBURG, VIRGINIA

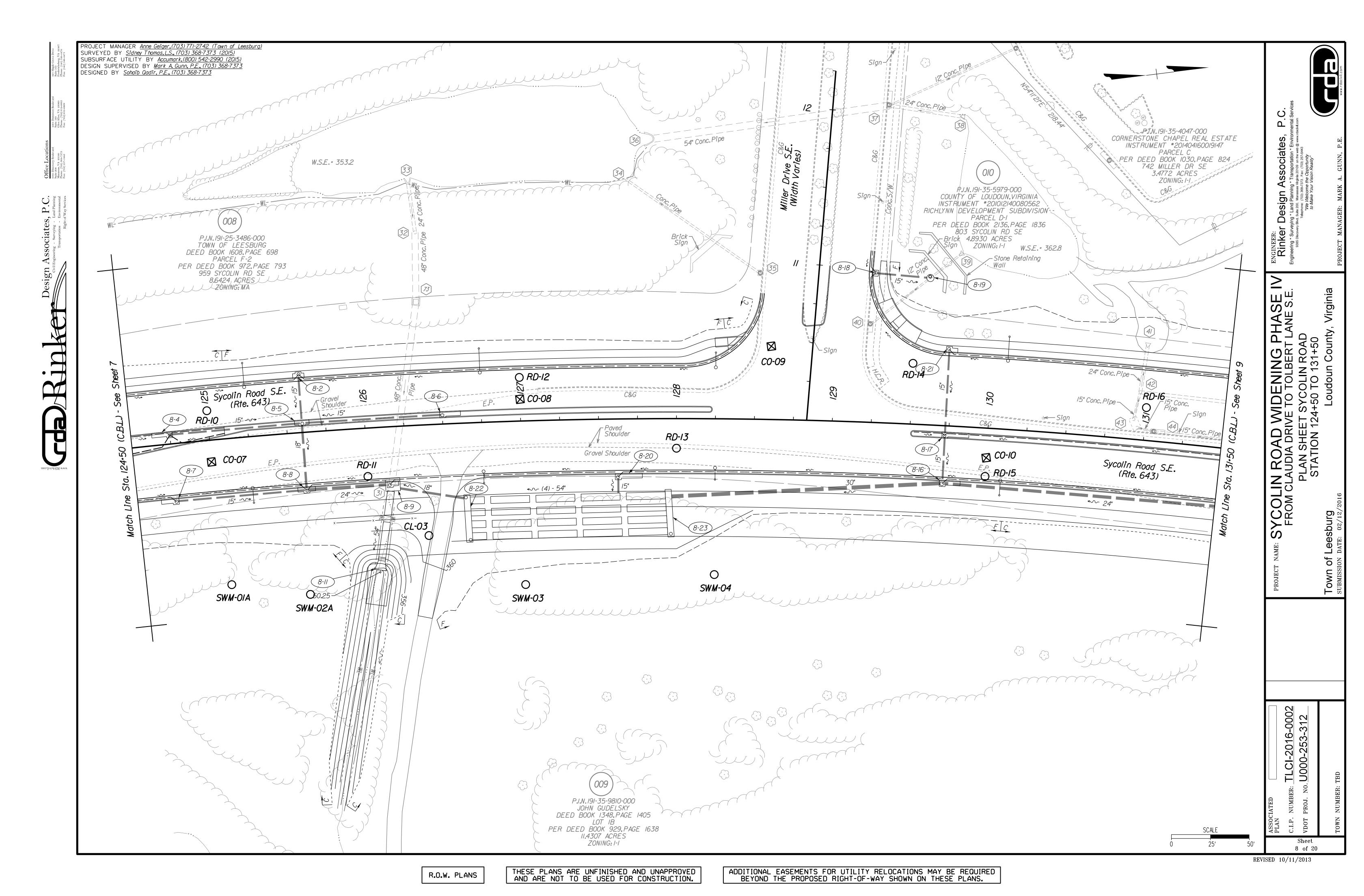
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SCALE: 1"=2000'	CHECKED BY: PZ	FIGURE NO.:

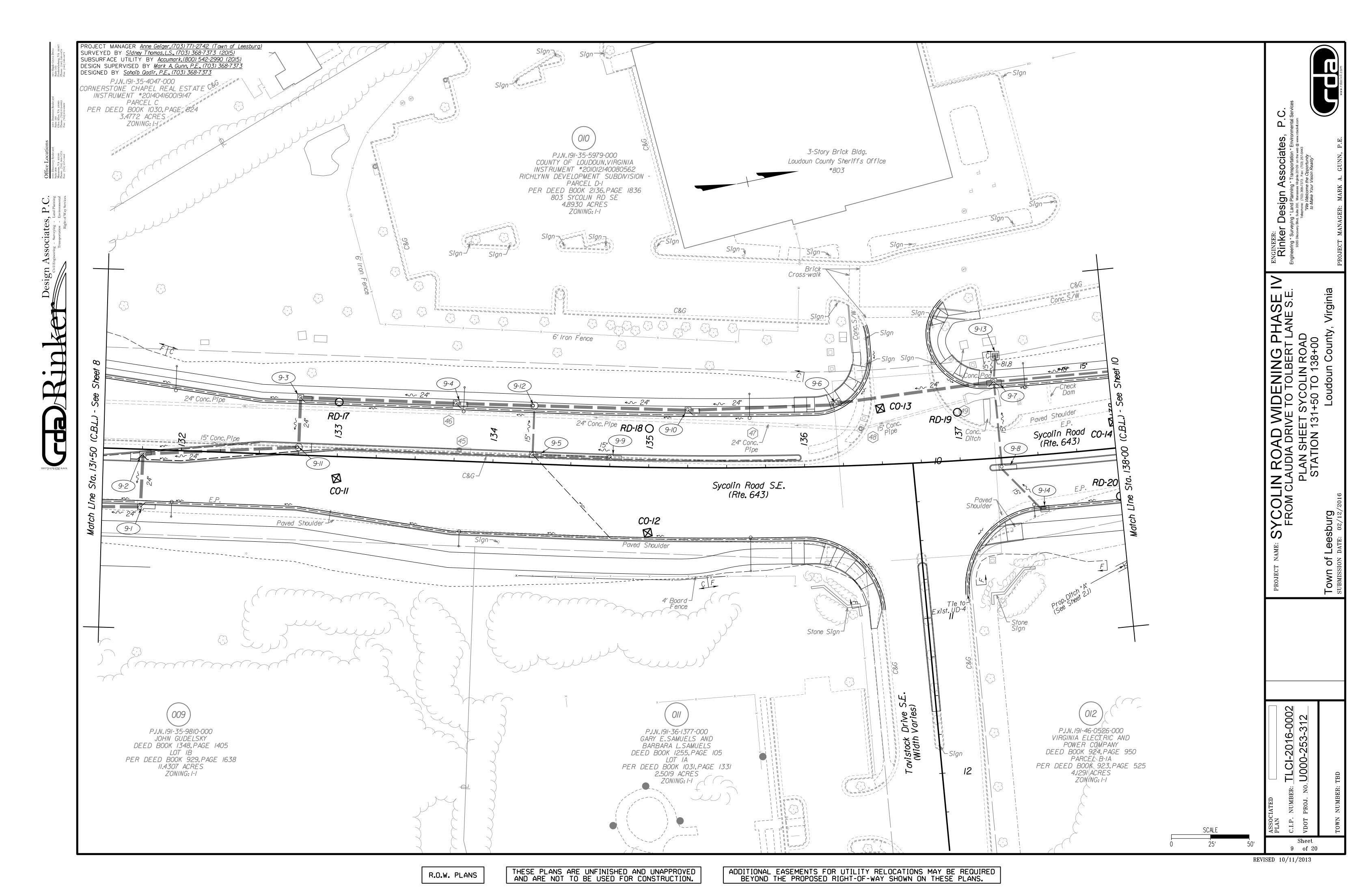
R.O.W. PLANS

THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR CONSTRUCTION.

R.O.W. PLANS

ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT-OF-WAY SHOWN ON THESE PLANS.





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THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR CONSTRUCTION.

R.O.W. PLANS

PROJECT MANAGER <u>Anne Geiger,(703) 771-2742 (Town of Leesburg)</u>
SURVEYED BY <u>Sidney Thomas, L.S., (703) 368-7373 (2015)</u>
SUBSURFACE UTILITY BY <u>Accumark,(800) 542-2990 (2015)</u> DESIGN SUPERVISED BY <u>Mark A. Gunn, P.E., (703) 368-7373</u> DESIGNED BY <u>Sohaib Qadir, P.E., (703) 368-7373</u> Associates, P.J.N.190-15-5301-000 THE NATIONAL INSTITUTE FOR AUTOMOTIVE SERVICE EXCELLENCE DEED BOOK 1809, PAGE 686 CORRECTED Design IN DEED BOOK 1864, PAGE 1008 AIRPORT COMMERCE PARK - LOT 3 PER INSTRUMENT \*200408200088207 IO5 BLUE SEAL DR SE II.9745 ACRES ZONING: I-I | Exist.UD-4 | Conc.S/W | Sign | Sign | I5" Conc.Pipe O RD-27 CO-18 🛭 <del>---</del>------**→** 15" ~> /5" ~~**>** Sycolin Road [11-4] (Rte. 643) RD-28 Tie to¬ Exist.UD-4 24" Conc. Pipe Sign RD-30 (mmmmmmmmm ¿www.www. └Pond Str. I-Story Brick & Stucco Bldg. Dominion Virginia Power \*620 012 P.I.N. 190-16-1332-000 P.I.N. 190-15-8432-000 LONG LANE ASSOCIATES AMERICAN NATIONAL RED CROSS P.J.N. 191-46-1875-000 LIMITED PARTNERSHIP INSTRUMENT \*200605020038835 AS VIRGINIA ELECTRIC AND DEED BOOK 1160, PAGE 1449 RERECORDED HIGH POINT - PARCEL IA IN INSTRUMENT \*200606010048446 POWER COMPANY DEED BOOK 924, PAGE 945 PER DEED BOOK 1105, PAGE 1105 HIGH POINT - LOT I 620 SYCOLIN RD SE 501TOLBERT LA SE PER DEED BOOK 1175, PAGE 1249 9.8973 ACRES 5.3000 ACRES 2.1951 ACRES ZONING: PEC ZONING: 1-1 ZONING: PEC SCALE 11 of 20 REVISED 10/11/2013 THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR CONSTRUCTION. ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT-OF-WAY SHOWN ON THESE PLANS.

R.O.W. PLANS

# APPENDIX B FIELD OPERATIONS

## Table B1 - Summary of Pavement Core Information Sycolin Road Widening PHASE IV Leesburg, Virginia

VDOT No. U000-253-312 P101, R201, C501; DMY No. 01.02095.01

Core Number	Station	Offset	Lane ID	AC Thickness (inches)	HCC Thickness (inches)	OGDL Thickness (inches)	CTA Thickness (inches)	GAB Thickness (inches)	Remarks
Sycolin Road									
CO-01	114+00	R 24	NB Lane	11.5	-	-	-	13.5	
CO-02	117+00	L 10	SB Lane	12.0	-	-	-	13.5	
CO-03	119+00	L 4	SB Acceleration Lane	8.0	-	-	-	14.0	
CO-04	119+55	R 28	NB RTL	9.5	-	-	-	12.5	
CO-06	121+00	R 17	SB Lane	12.0	-	-	-	14.0	
CO-07	125+00	R 14	NB Lane	7.5	-	-	-	13.0	
CO-08	127+00	L 14	SB Acceleration Lane	8.5	-	-	-	10.0	
CO-10	130+00	R 19	NB Lane	8.5	-	-	-	13.0	
CO-11	133+00	R 14	Median Stripe Zone	8.5	-	-	-	11.0	
CO-12	135+00	R 42	NB RTL	9.5	-	-	-	12.0	
CO-14	138+00	L 17	SB RTL	12.0	-	-	-	11.0	
CO-15	141+00	L 4	Median Stripe Zone	11.0	-	-	-	10.0	
CO-16	142+00	R 17	NB RTL	9.5	-	-	-	13.0	
CO-17	144+00	L 4	SB Lane	7.0	-	-	-	6.0	
CO-18	146+00	L 6	Median Stripe Zone	9.0	-	-	-	16.0	
CO-19	147+50	L 22	SB Lane	11.0	-	-	-	17.0	
Unnamed Cross Road	1			•			•		
CO-05	120+35	L 25	WB RTL	4.5	-	-	-	10.0	unnamed road turning into the airport
Miller Drive SE	·								
CO-09	128+60	L 47	EB RTL	10.0	-	-	-	11.0	
Dominion Power Entrance	- ' }								
CO-13	136+50	L 36	EB RTL	13.0	-	-	-	12.0	

#### Table B2 - Summary of Pavement Subgrade Soil Conditions

Sycolin Road Widening PHASE IV, Leesburg, VA VDOT No. U000-253-312 P101, R201, C501; DMY No. 01.02095.01

Boring Information		Subgrade Soils Characteristics																		
Boring ID	Station	Offset	Subgrade Soil Depth (feet)	Soil Class.	Fine Content (%)	LL (%)	PI (%)	CBR	Reference CBR/Proctor Tests	Swell (%)	OMC (%)	NMC (%)	SPT N-Value	High Plasticity (CH/MH)	Low CBR (CBR < 6.7)	Swell >5%	Soft/ Very Loose	Excessive Moisture (NMC >1.2 OMC)	Highly Organic	Subgrade Treatment Option
Sycolin Road																				
RD-01	114+00	R 36	2-5'	SC	26.4	29	13	18.5	-	0.5	10.8	12.4	45	-	-	-	-	-	-	
RD-02	115+00	L 28	2-5'	CL-ML	54.3	25	4	14.5	RD-06	1.1	13	11.2	27	-	-	-	-	-	-	
RD-03	116+00	R 26	2-5'	SC	-	-	-	18.5	RD-01	0.5	10.8	9.4	79	-	-	-	-	-	-	
RD-04	117+00	L 28	1-4'	GC-GM	31.5	26	6	18.5	RD-01	0.5	10.8	10.6	15	-	-	-	-	-	-	<u> </u>
RD-05	118+00	R 25	2-5'	GC	18.3	28	9	18.5	RD-01	0.5	10.8	9.4	100	-	-	-	-	-	-	
RD-06	119+00	L 25	1-4'	CL	60.3	27	10	14.5	-	1.1	13	20.7	8	-	-	-	-	Yes	-	A, B or C
RD-07	121+00	L 25	0-3'	ML	-	-	-	14.5	RD-06	1.1	13	4.4	100	-	-	-	-	-	-	
RD-08	123+00	L 25	0-3'	SC	-	-	-	18.5	RD-01	0.5	10.8	11.2	11	-	-	-	-	-	-	
RD-09	124+00	R 31	2-5'	GC	20.3	25	8	18.5	RD-01	0.5	10.8	5.9	100	-	-	-	-	-	-	<u> </u>
RD-10	125+00	L 19	2-5'	SM	-	-	-	18.5	RD-01	0.5	10.8	38.1	12	-	-	-	-	Yes	-	A, B or C
RD-11	126+00	R 31	0-3'	ML	-	-	-	14.5	RD-06	1.1	13	10	9	-	-	-	-	-	-	
RD-12	127+00	L 28	2-5'	CL	-	-	-	14.5	RD-06	1.1	13	17.2	12	-	-	-	-	Yes	-	A, B or C
RD-13	128+00	R 19	1-4'	CL	-	-	-	14.5	RD-06	1.1	13	33.9	8	-	-	-	-	Yes	-	A, B or C
RD-14	129+50	L 39	0-3'	ML	-	-	-	14.5	RD-06	1.1	13	9.3	15	-	-	-	-	-	-	
RD-15	130+00	R 31	2-5'	SM	-	-	-	18.5	RD-01	0.5	10.8	26.9	15	-	-	-	-	Yes	-	A, B or C
RD-16	131+00	L 21	0-3'	SM	-	-	-	18.5	RD-01	0.5	10.8	7.7	22	-	-	-	-	-	-	
RD-17	133+00	L 35	6-9'	AR @ 2.5'	-	-	-	-	-	-	-	-	100	-	-	-	-	-	-	
RD-18	135+00	L 25	4-7'	AR @ 4.5'	-	-	-	-	-	-	-	-	100	-	-	-	-	-	-	
RD-19	137+00	L 31	2-5'	CL	58.8	35	15	14.5	RD-06	1.1	13	17.8	8	-	-	-	-	Yes	-	A, B or C
RD-20	138+00	R 31	1-4'	GC	32.8	25	8	22.7	RD-22	0.8	9.4	13.1	9	-	-	-	-	Yes	-	A, B or C
RD-21	139+00	L 36	2-5'	AR @ 4.5'	-	-	-	-	-	-	-	-	100	-	-	-	-	-	-	
RD-22	140+00	R 28	0-3'	SC	43.3	33	15	22.7	-	0.8	9.4	7.5	9	-	-	-	-	-	-	
RD-23	141+00	L 28	2-5'	SM	48.8	43	16	22.7	RD-22	0.8	9.4	17.8	17	-	-	-	-	Yes	-	A, B or C
RD-24	142+00	R 45	5-8'	SM	39.5	38	13	22.7	RD-22	0.8	9.4	16.1	46	-	-	-	-	Yes	-	A, B or C
RD-25	143+00	L 24	2-5'	SC	17.1	31	11	17.1	RD-27	0.3	10.4	10.6	56	-	-	-	-	-	-	
RD-26	144+00	R 28	3-6'	MH	61.5	68	18	-	-	-	-	31.2	7	Yes	-	-	-	-	-	D
RD-27	145+00	L 28	2-5'	SC	44.1	28	10	17.1	-	0.3	10.4	7.4	13	-	-	-	-	-	-	
RD-28	146+00	R 31	1-4'	SC-SM	22.8	25	4	17.1	RD-27	0.3	10.4	6.2	14	-	-	-	-	-	-	<u> </u>
RD-29	147+00	L 31	2-5'	СН	-	-	-	-	-	-	-	35.2	8	Yes	-	-	-	-	-	D
RD-30	147+00	R 55	6-9'	AR @ 4.1'	-	-	-	-	-	-	-	-	100	-	-	-	-	-	-	Al .

- A. For unstable areas as identified by proofrolling, dry up subgrade soils using discing or other mechanical methods to within 120% of the optimum moisture content for a minimum of 2 feet and recompact in-place.
- B. For unstable areas as identified by proofrolling, dry up subgrade soils using quicklime to within 120% of the optimum moisture content for a minimum of 2 feet and recompact in-place.
- C. For unstable areas as identified by proofrolling, undercut to a depth of at least 2 feet below subgrade and backfill with compacted suitable fill with a minimum CBR value of 10.
- D. Undercut to a depth of at least 2 feet below subgrade and backfill with Select Material Type I with a minimum CBR value of 30 on a layer of woven subgrade stabilization geotextile; or undercut to a depth of at least 3 feet below subgrade, then backfill with compacted suitable fill with a minimum CBR value of 10.



Core CO-01 Sycolin Road, Station 114+00, R 24, Northbound Lane



Core CO-02 Sycolin Road, Station 117+00, L 10, Southbound Lane



Core CO-03 Sycolin Road, Station 119+00, L 4, Southbound Acceleration Lane



Core CO-04
Sycolin Road, Station 119+55, R 28, Northbound Right Turn Lane



Core CO-05
Sycolin Road, Station 120+35, L 25, Cross Road (no name)



Sycolin Road, Station 121+00, R 17, Southbound Lane



Core CO-07 Sycolin Road, Station 125+00, R 14, Northbound Lane



Core CO-08 Sycolin Road, Station 127+00, L 14, Southbound Acceleration Lane



Core CO-09
Miller Drive, Station 128+60, L 47, Eastbound, Right Turn Lane



Core CO-10 Sycolin Road, Station 130+00, R 19, Northbound Lane



Core CO-11
Sycolin Road, Station 133+00, R 14, Median Stripe Zone



Core CO-12 Sycolin Road, Station 135+50, R 42, Northbound, Right Turn Lane



Core CO-13

Dominion Power Entrance, Station 136+50, L 36, Eastbound, Right Turn Lane



Sycoolin Road, Station 138+00, L 17, Southbound, Right Turn Lane



Core CO-15

Sycolin Road, Station 141+00, L 4, Median Stripe Zone



Core CO-16

Sycolin Road, Station 142+00, R 17, Northbound, Right Turn Lane

Sycolin Road Widening PHASE IV, Leesburg, Virginia VDOT Project No. U000-253-312 P101, R201, C501; DMY No. 01.02095.01 Appendix B



Core CO-17 Sycolin Road, Station 144+00, L 4, Southbound Lane



Core CO-18
Sycolin Road, Station 146+00, L 6, Median Stripe Zone

Sycolin Road Widening PHASE IV, Leesburg, Virginia VDOT Project No. U000-253-312 P101, R201, C501; DMY No. 01.02095.01 Appendix B



Sycolin Road, Station 147+50, L 22, Southbound Lane

#### REFERENCE NOTES FOR BORING LOGS

# **I. Drilling and Sampling Symbols:**

SS	-	Split Spoon Sampler	RB	-	Rock Bit Drilling
ST	-	Shelby Tube Sampler	BS	-	Bulk Sample of Cuttings
RC	-	Rock Core; NX, BX, AX	PA	-	Power Auger (no sample)
PM	-	Pressuremeter	HSA	-	Hollow Stem Auger
DC	-	Dutch Cone Penetrometer	WS	-	Wash Sample

Standard Penetration Test (SPT) resistance refers to the blows per foot (bpf) of a 140 lb hammer falling 30 inches on a 2 in. O.D. split-spoon sampler as specified in ASTM D-1586. The blow count is commonly referred to as the N-value.

## **II. Correlation of Penetration Resistances to Soil Properties:**

Relative Density of Cohesionless Soils Consistency of Cohesive Soils

SPT-N (bpf)	Relative Density	SPT-N (bpf)	Consistency
0 - 3 4 - 9 10 - 29 30 - 50 >50	Very Loose Loose Medium Dense Dense Very Dense	0 - 1 2 - 4 5 - 8 9 - 15 16 - 30 31 - 50 >50	Very Soft Soft Firm Stiff Very Stiff Hard Very Hard

Weathered Rock (WR) may be defined as SPT-N values exceeding 60 bpf depending on site specific conditions. Refer carefully to boring logs.

Rock Fragments, gravel, cobbles, boulders, or debris may produce N-values that are not representative of actual soil properties.

### III. Unified Soil Classification Symbols:

SC – Clayey Sands

GP – Poorly Graded Gravel	ML – Low Plasticity Silts
GW – Well Graded Gravel	MH – High Plasticity Silts
GM – Silty Gravel	CL – Low Plasticity Clays
GC – Clayey Gravels	CH – High Plasticity Clays
SP – Poorly Graded Sands	OL – Low Plasticity Organics
SW – Well Graded Sands	OH – High Plasticity Organics
SM – Silty Sands	CL-ML – Dual Classification (Typical)

### IV. Laboratory Testing and Water Level Symbols:

LL - LIQUID LIMIT (%) PI - PLASTIC INDEX (%)	Water Level at Time     Drilling, or as Shown
W - MOISTURE CONTENT (%) DD - DRY DENSITY (PCF)	▼ Water Level at End of Drilling, or as Shown
NP - NON PLASTIC -200 - PERCENT PASSING NO. 200 SIEVE PP - POCKET PENETROMETER (TSF)	₩ Water Level After 24 Hours, or as Shown



# UNIFIED SOIL CLASSIFICATION SYSTEM

#### UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART

COARSE-GRAINED SOILS

(more than 50% of material is larger than No. 200 sieve size.)

Olcan Graveis (Ecss than 570 lines)	Clean Gravels	Less than 5% fines)
-------------------------------------	---------------	---------------------

		nean (	stavels (Less than 5% lines)
GRAVELS		GW	Well-graded gravels, gravel-sand mixtures, little or no fines
More than 50% of coarse		GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
fraction larger	G	ravels	s with fines (More than 12% fines)
than No. 4 sieve size		GM	Silty gravels, gravel-sand-silt mixtures
		GC	Clayey gravels, gravel-sand-clay mixtures
	С	lean S	Sands (Less than 5% fines)
SANDS		SW	Well-graded sands, gravelly sands, little or no fines
50% or more of coarse		SP	Poorly graded sands, gravelly sands, little or no fines
fraction smaller	S	ands	with fines (More than 12% fines)
than No. 4 sieve size		SM	Silty sands, sand-silt mixtures
		SC	Clayey sands, sand-clay mixtures

#### **FINE-GRAINED SOILS**

(50% or more of material is smaller than No. 200 sieve size.)

SILTS AND		ML	Inorganic silts and very fine sands, rock flour, silty of clayey fine sands or clayey silts with slight plasticity
CLAYS Liquid limit less than		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
50%		OL	Organic silts and organic silty clays of low plasticity
SILTS		МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
CLAYS Liquid limit 50%		СН	Inorganic clays of high plasticity, fat clays
or greater		ОН	Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS	77. 77. 77.	PT	Peat and other highly organic soils

#### LABORATORY CLASSIFICATION CRITERIA

GW  $C_u = \frac{D_{60}}{D_{10}}$  greater than 4;  $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$  between 1 and 3

GP Not meeting all gradation requirements for GW

GM Atterberg limits below "A" line or P.I. less than 4

GC Atterberg limits above "A" line with P.I. greater than 7

Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols

SW  $C_u = \frac{D_{60}}{D_{10}}$  greater than 4;  $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$  between 1 and 3

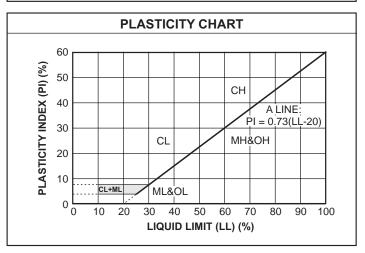
SP Not meeting all gradation requirements for GW

SM Atterberg limits below "A" line or P.I. less than 4

SC Atterberg limits above "A" line with P.I. greater than 7

Limits plotting in shaded zone with P.I. between 4 and 7 are borderline cases requiring use of dual symbols.

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:





# MATERIAL AND SAMPLE SYMBOLS LIST

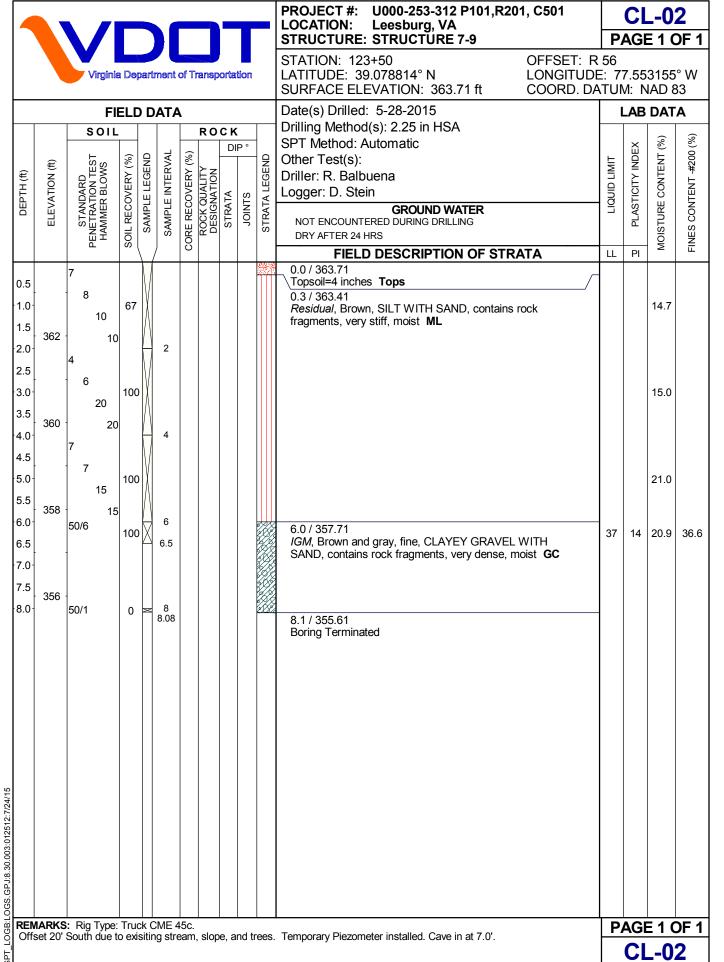


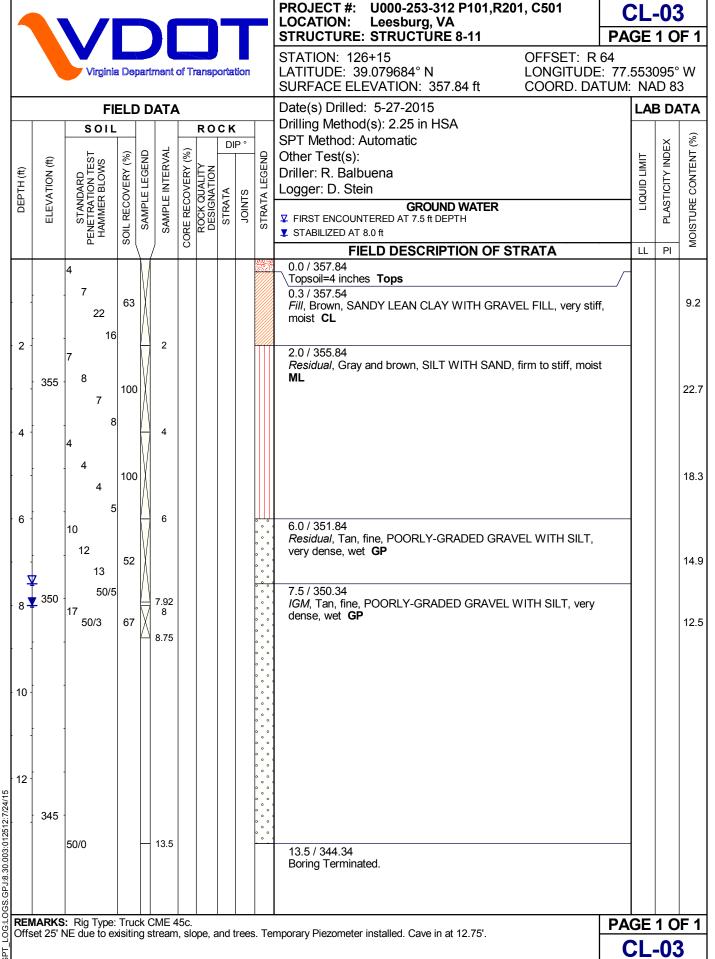


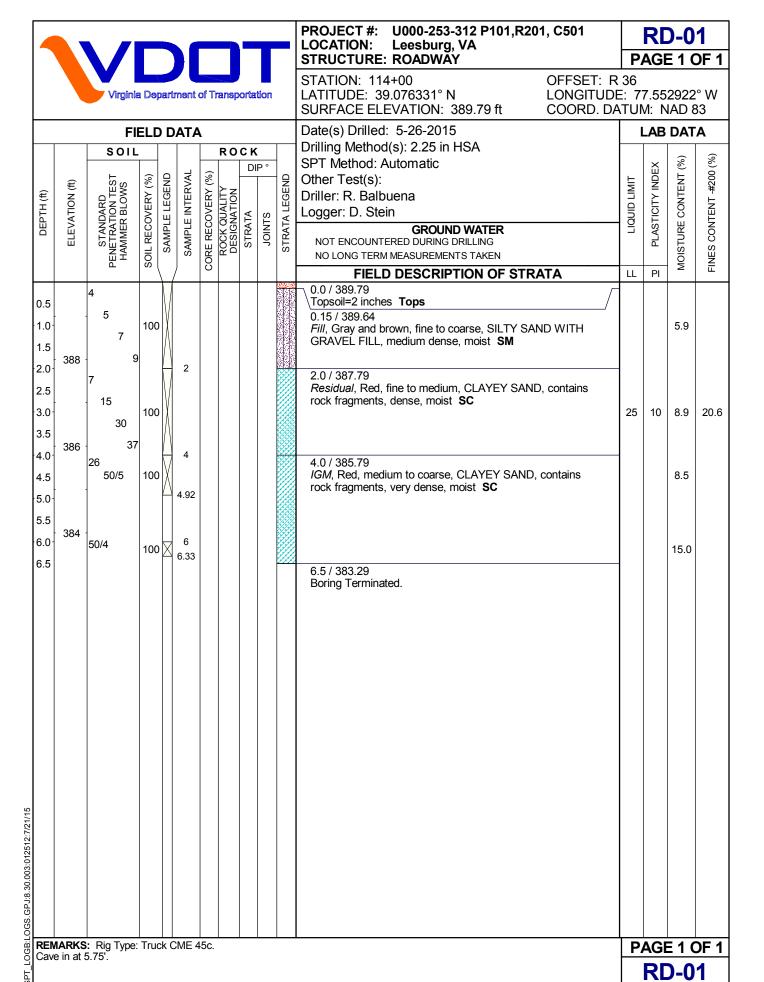
# MATERIAL AND SAMPLE SYMBOLS LIST

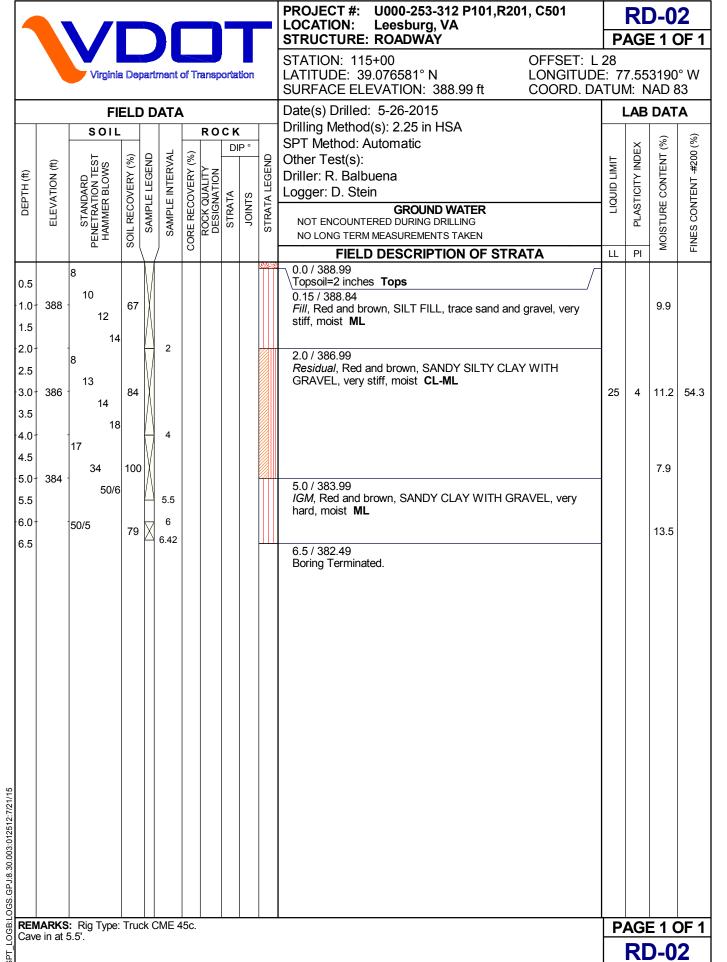
		STWIDOLS	LIST	
Pavement/Soils	Sedimentary Rocks	Igneous Rocks	Metamorphic Rocks	Sampling
TOPS- TOPSOIL SC/CH CH/CL CH/MH CH/SC  TOPSOIL SC/CH CH/SC/CH/SC  TOPSOIL SC/CH CH/SC  TOPSOIL SC/CH CH/SC/CH/SC  TOPSOIL SC/CH CH/SC  TOPSOIL SC/CH CH/S	BLD-Boulder Bed  0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	CHT Charnocktite	MSLS Metasiltstone	
CL/ML CL/SC CL/CH GP/GW Crushed Aggregate	Dolostone LST-DLS-		MSST Metasandstone	
GW/GP ML/MH GC/SC OH/OL GP/SP	Interbeded Limestone/Dolostone		Quartzite	
OL/OH Peat Organic SC/CL Organic  V V V V V V  V V V V V V  V V V V V V	CHRT  TATALATATA  TATALATATA  TATALATATA  TATALATATATAT		SPS Soapstone ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈ ≈	
SC/GC SC-SM SP/SW SM/GM SM/MH			MBST Metabasalt	
SM/ML SM/SC SP/GP SW/SP			MBL Marble	

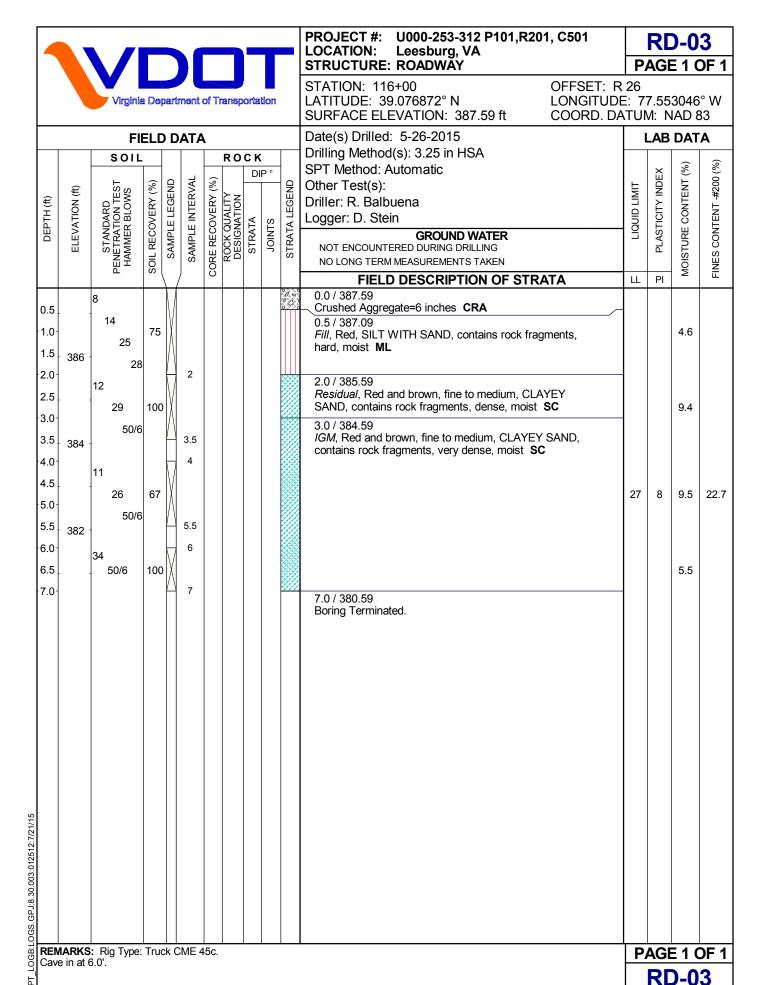
	•		1						PROJECT #: U000-253-312 P101,R201, C501 LOCATION: Leesburg, VA		CL	0	1
		<b>\</b> /							STRUCTURE: STRUCTURE 7-9	P	AGI	E 1 (	OF 1
		Minninia	Dono	<b>A</b>	of To		4-41-r		STATION: 123+30 OFFSET: L LATITUDE: 39.078856° N LONGITUDE		7 55	2504	0 \A/
		Virginia	a Depa	riment	OFIR	anspor	Taluon	l	SURFACE ELEVATION: 363.21 ft COORD. DA				
		FI	ELD I	DATA	١				Date(s) Drilled: 5-22-2015		LAB	DAT	Ά
		SOIL				ROC			Drilling Method(s): 2.25 in HSA			<u> </u>	(9)
	$\overline{}$	<u> </u>	(%) E	¥ (	(%)	-	DIP °		SPT Method: Automatic Other Test(s):		PLASTICITY INDEX	MOISTURE CONTENT (%)	FINES CONTENT -#200 (%)
€	NC E	SD TES	\rac{A}{2}   \frac{A}{2}   \		ERY (	ΣΝ		GEN	Driller: M. Vargas	I I	≦	NTE	¥   <u>+</u>
DEPTH (ft)	ELEVATION (ft)	ADAR TION R BL			SOVE	NA I	TS ST	TA LE	Logger: D. Stein	LIQUID LIMIT	TICI	E C0	Z EN
	ELE	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	JOINTS	STRATA LEGEND	GROUND WATER  NOT ENCOUNTERED DURING DRILLING	=	PLA8	TUR	000
		PEN	SOIL	S S	SORE	۵۵		00	DRY AFTER 24 HRS			MOIS	l NES
			$\rightarrow$	$\leftarrow$	-	+	+	- P. S.	FIELD DESCRIPTION OF STRATA  0.0 / 363.21	LL	PI		
		6	1						\Topsoil=3 inches TOPS				
-		16	100						0.25 / 362.96  Fill, Gray and black, fine to coarse, SILTY SAND WITH			6.0	
Ī		22		V					GRAVEL FILL, dense, moist SM				
2 -		18		2					2.0 / 361.21				
İ		3							Residual, Gray, SILT, trace sand, firm, moist ML				
-	360	3	100									20.5	
Ī	300	3		V									
4 -		3	<u>                                     </u>	4					4.0 / 359.21				
		13	1						Residual, Brown and gray, FAT CLAY WITH SAND, trace gravel, firm, moist CH				
-		3 _	100						gravei, iirm, moist <b>Ch</b>			22.6	
		5	. /	\									
6		5	) <u>{:</u>	6									
		2											
-		4	100							56	30	23.9	71.
		4	. /	\									
8	355	8	]	8				833	8.0 / 355.21				
	000	9							Residual, White and brown, fine to coarse, CLAYEY GRAVEL WITH SAND, medium dense, moist <b>GC</b>				
-		18	100						GIVVEE WITH OAVE, median dense, most 30	27	13	7.6	16.
		19											
10		]	1 1	10									
12 -		-											
									12.5 / 350.71				
-	350								IGM, White and brown, fine to coarse, CLAYEY GRAVEL WITH SAND, very dense, moist GC				
		50/0		13.5				XXX)	13.5 / 349.71				
									Boring Terminated				
REN	IARKS	Rig Type:	Truck	CME	45c.	at 4.4.1	 '	1	<u> </u>	P	AGI	E 1 (	DF '
ıem	porary	Piezometer	ınstalle	eu. Ca	ve in	at 11.(	J.					0	

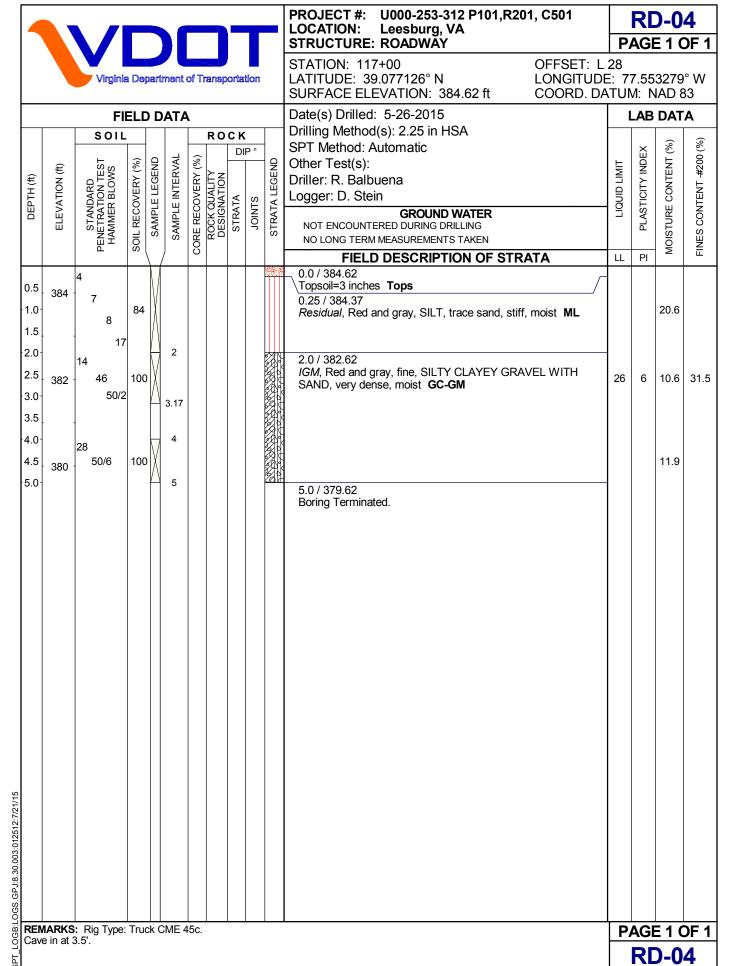


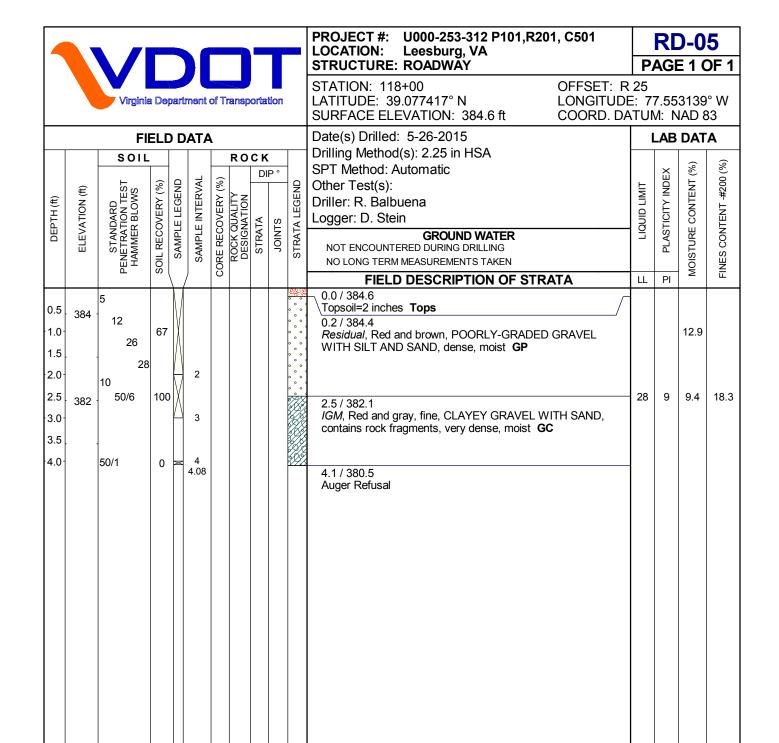








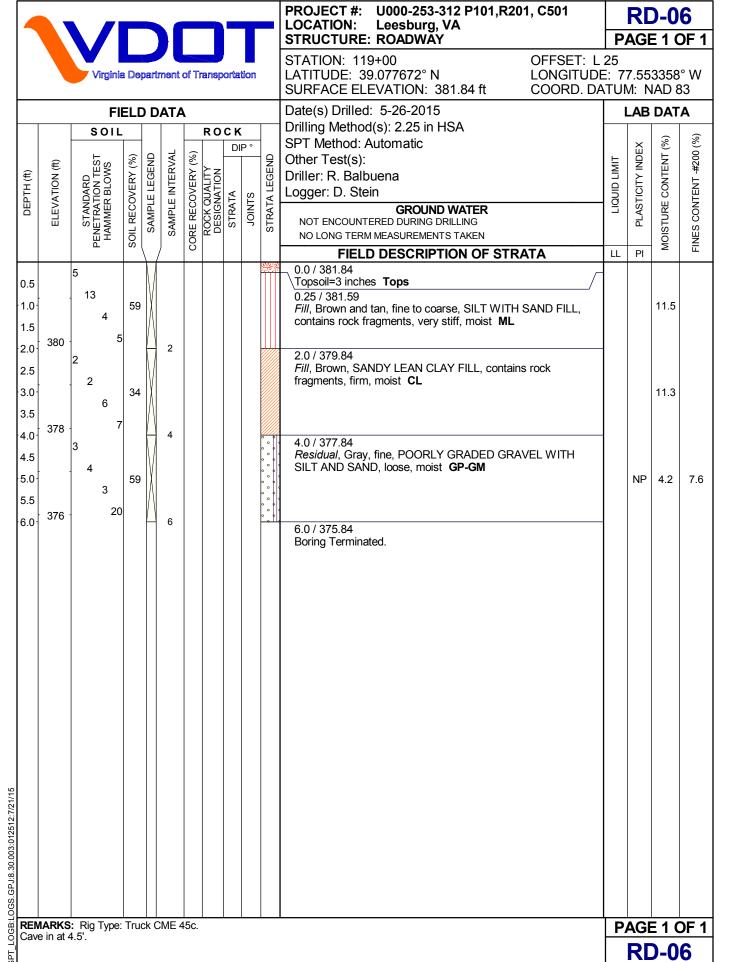


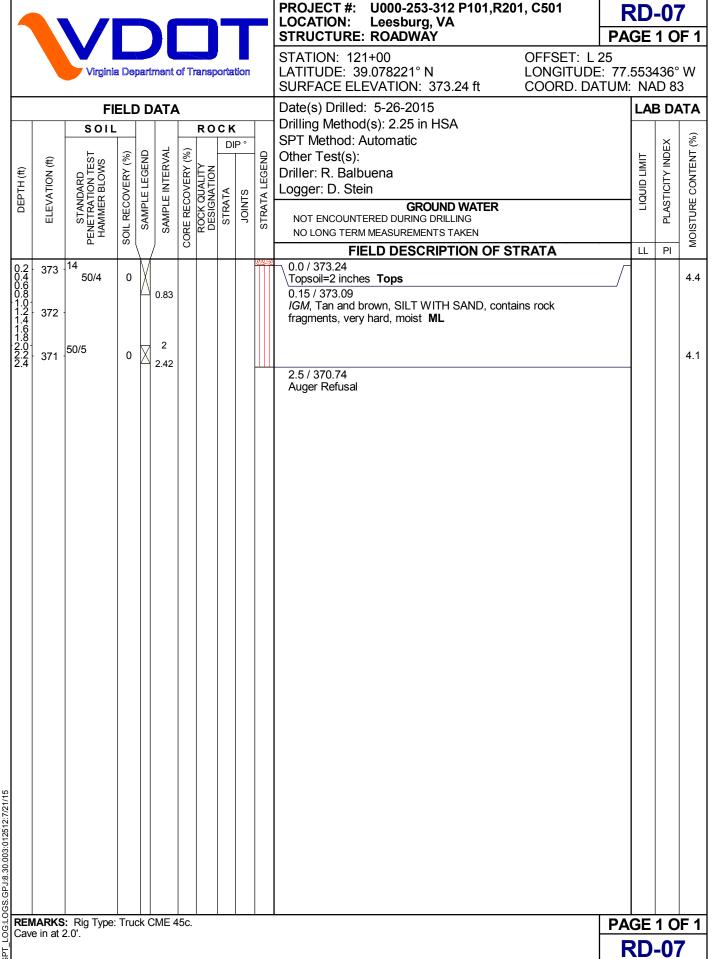


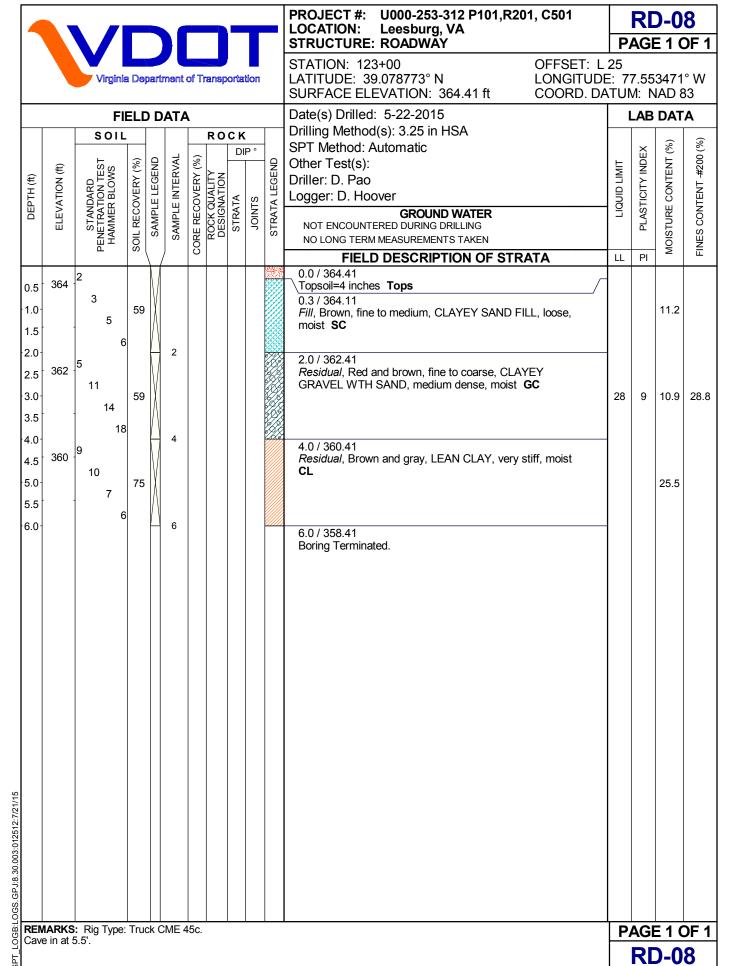
**REMARKS:** Rig Type: Truck CME 45c. Cave in at 3.5'.

PAGE 1 OF 1 RD-05

.OGS.GPJ:8.30.003:012512:7/21/15









PROJECT #: U000-253-312 P101,R201, C501

LOCATION: Leesburg, VA STRUCTURE: ROADWAY

**RD-09**PAGE 1 OF 1

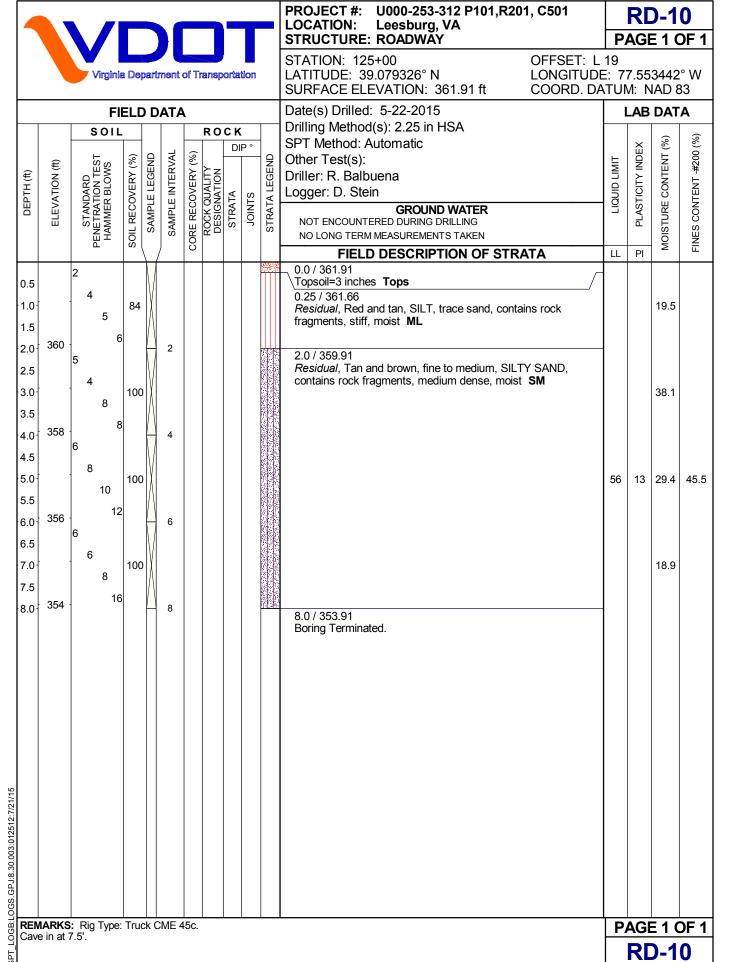
PAGE 1 OF 1 RD-09

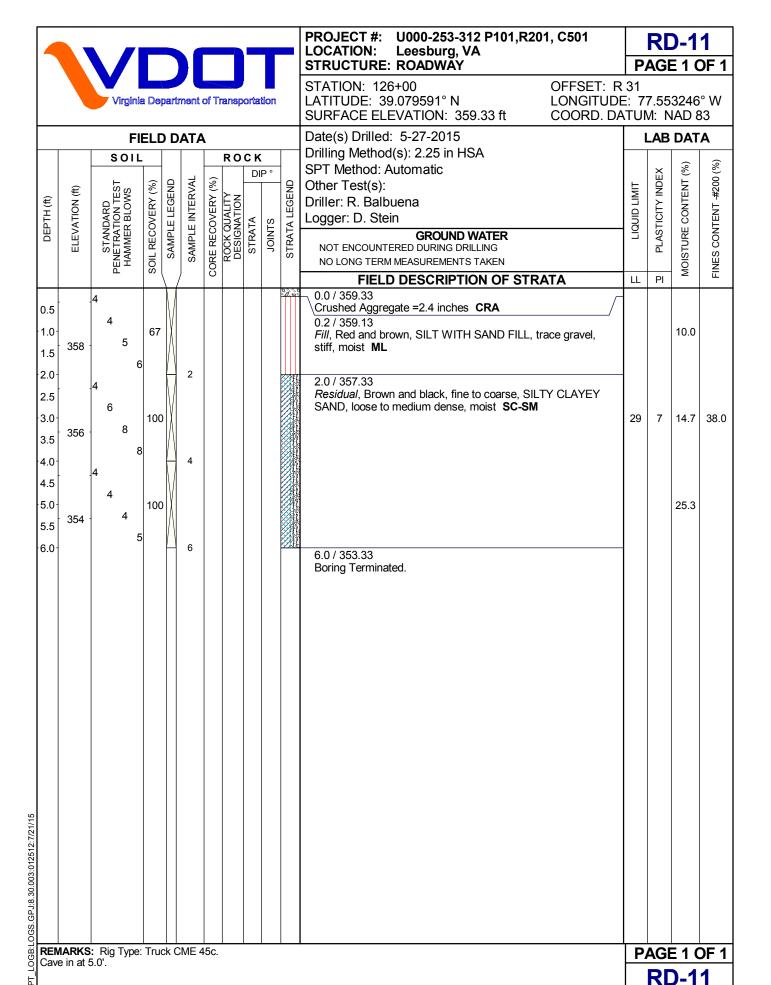
STATION: 124+00 OFFSET: R 31

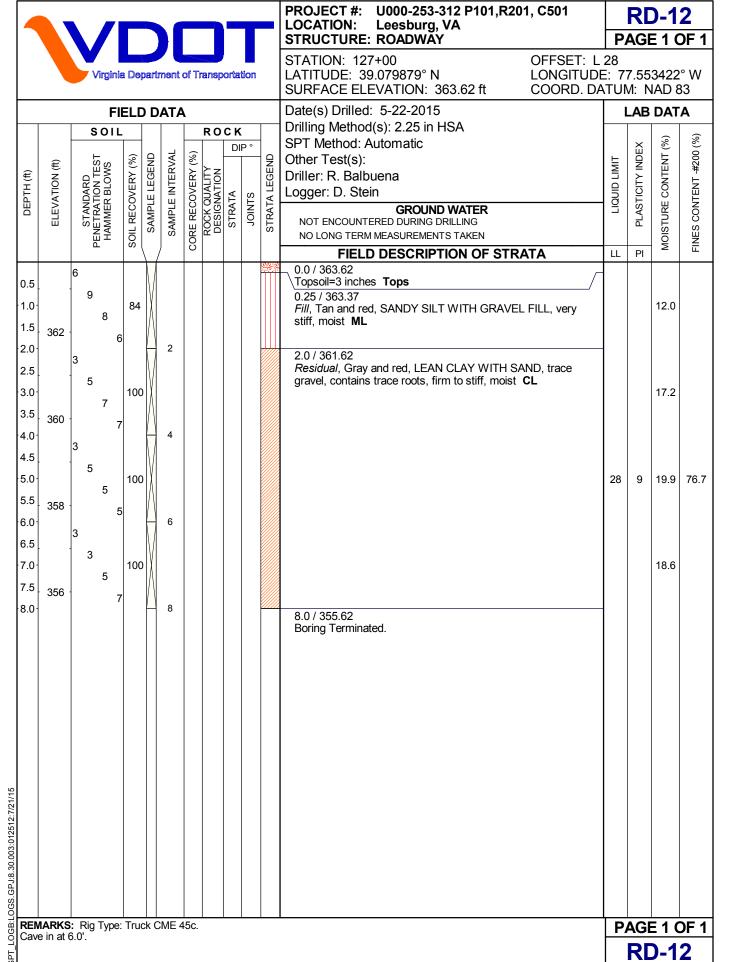
LATITUDE: 39.079048° N LONGITUDE: 77.553276° W SURFACE ELEVATION: 361.94 ft COORD. DATUM: NAD 83

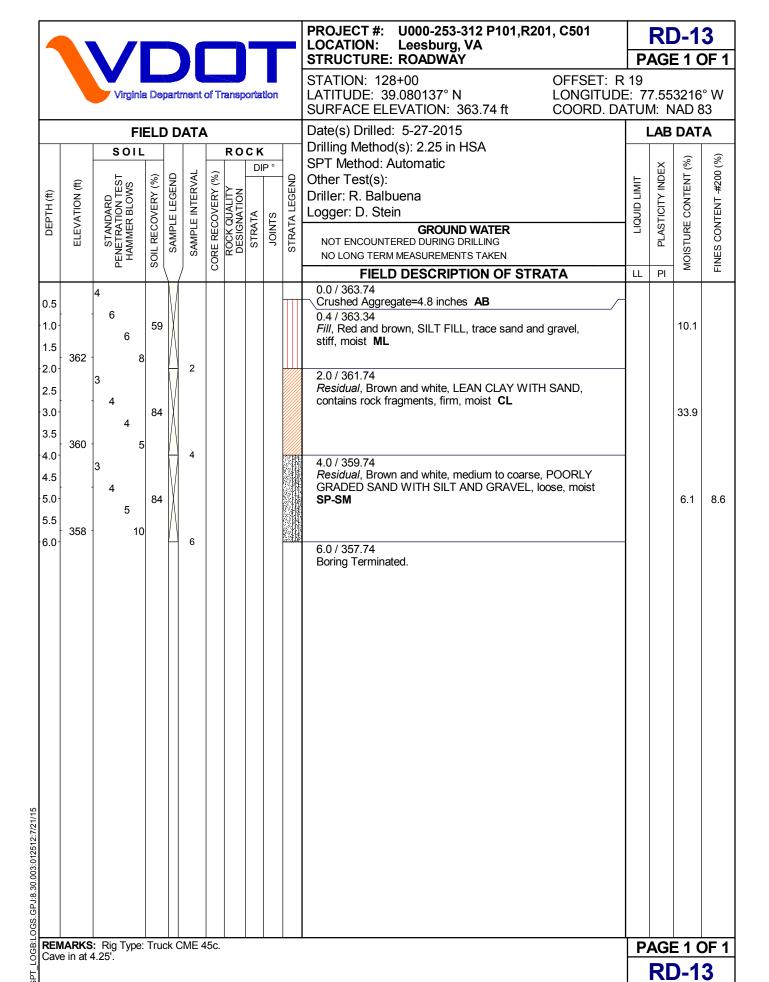
		· · · · · · · · · · · · · · · · · · ·	- 50			· · · ·				SURFACE ELEVATION: 361.94 ft COORD. DA	ÜŢÚſ	и: N	IAD 8	33
		FII	ELC	D (	ATA	\				Date(s) Drilled: 5-27-2015	ı	LAB	DAT	Α
		SOIL					RO			Drilling Method(s): 2.25 in HSA SPT Method: Automatic		_	(%	(%)
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	P STNIOL	STRATA LEGEND	Other Test(s): Driller: R. Balbuena Logger: D. Stein  GROUND WATER  NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	DISTURE CONTENT (%)	FINES CONTENT -#200 (%)
		8 4	8	\	)	8				FIELD DESCRIPTION OF STRATA	LL	PI	MOI	E N
0.5 1.0 1.5 2.0	-	4 8 12 30	100		2					0.0 / 361.94 Topsoil=3 inches <b>Tops</b> 0.25 / 361.69 Fill, Gray, SILTY SAND WITH GRAVEL FILL, medium dense, moist <b>SM</b>			8.9	
2.5 3.0 3.5	-	50/3	56	X	2.75					Residual, Gray and brown, fine, CLAYEY GRAVEL WITH SAND, very dense, dry GC  2.5 / 359.44  IGM, Gray and brown, fine, CLAYEY GRAVEL WTH	25	8	5.9	20.3
4.0	050	50/3	68	X	4 4.25					SAND, very dense, dry <b>GC</b> 4.3 / 357.64 Auger Refusal			6.5	
:LOGS.GPJ:8.30.003:012512:7/21/15														

**REMARKS:** Rig Type: Truck CME 45c. Cave in at 4.0'.

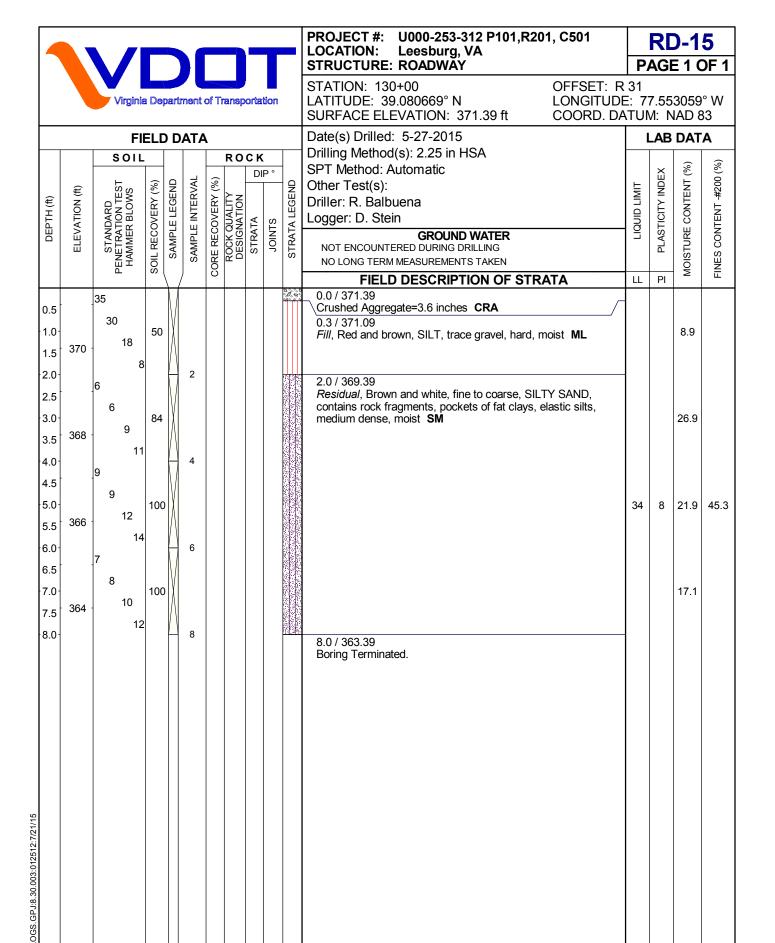






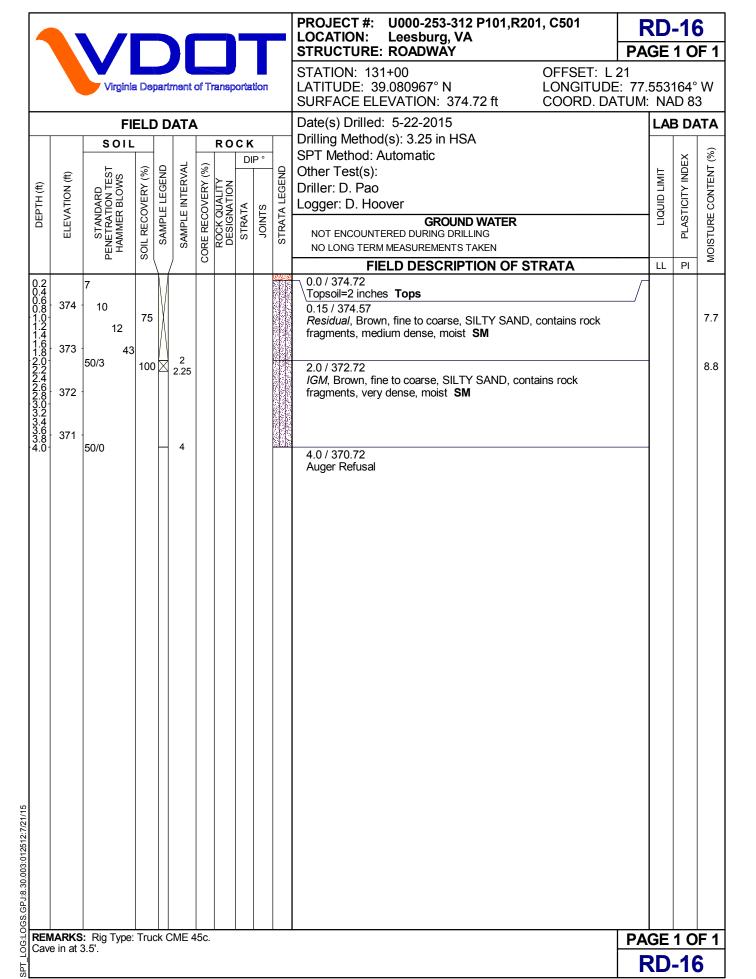


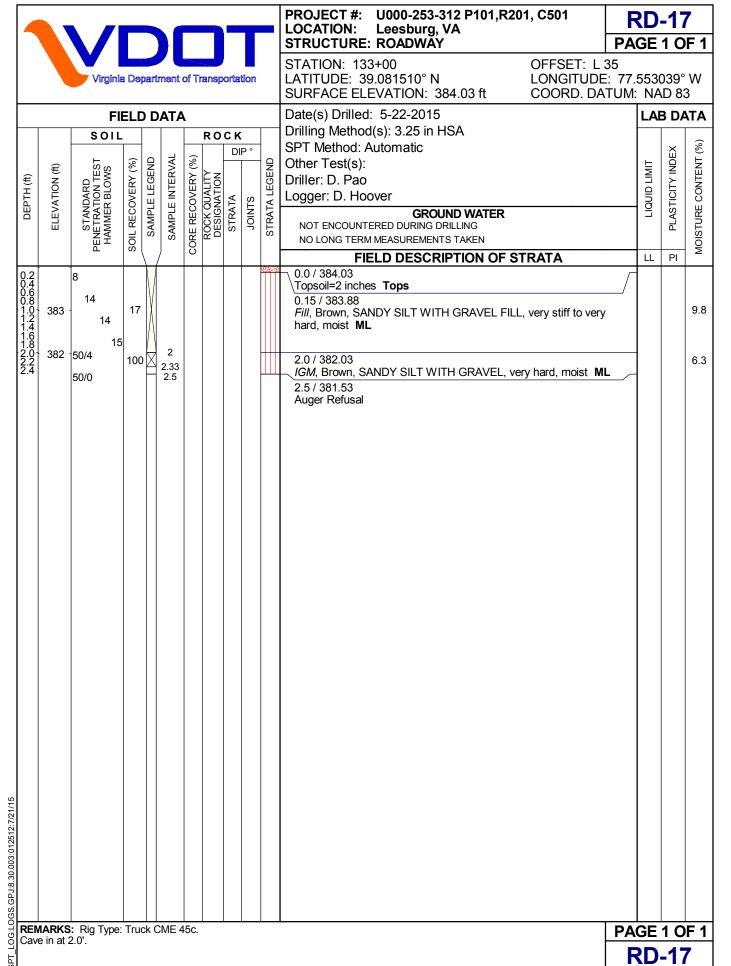
	•									PROJECT #: U000-253-312 P101,R201, C501 LOCATION: Leesburg, VA		RE	<b>)-1</b>	4
										STRUCTURE: ROADWAY			E 1 (	
		Virginia	a Depa	ırtment	of Tra	anspo	ortati	ion		STATION: 129+50 OFFSET: L LATITUDE: 39.080570° N LONGITUDE SURFACE ELEVATION: 369.96 ft COORD. DA	E: 7			
		FI	ELD	DATA	4					Date(s) Drilled: 5-22-2015	ı	_AB	DAT	Α
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS		SAMPLE INTERVAL	(%)	ROCK QUALITY DESIGNATION	DIF	SOINTS	STRATA LEGEND	Drilling Method(s): 3.25 in HSA SPT Method: Automatic Other Test(s): Driller: D. Pao Logger: D. Hoover  GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN  FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	□ PLASTICITY INDEX	MOISTURE CONTENT (%)	FINES CONTENT #200 (%)
0.5 1.0 1.5 2.0 2.5 3.0 4.0 5.0 5.5 6.0	368 366 364	4 5 10 16 13 25 40 50/5 28 50/3	87	3.92 4 4.75 6.17						O.0 / 369.96 Topsoil=2 inches Tops O.15 / 369.81 Fill, Red, SILT WITH GRAVEL FILL, trace sand, stiff, moist ML  2.0 / 367.96 Residual, Brown and white, fine to coarse, CLAYEY SAND, contains rock fragments, very dense, moist SC  3.5 / 366.46 IGM, Brown and white, fine to coarse, CLAYEY SAND, contains rock fragments, very dense, moist SC  6.2 / 363.76 Auger Refusal	28	9	9.3 9.4 5.0	22.5
Cave	e in at s	B: Rig Type: 5.5'.			45c.								≣ 1 ( <b>)-1</b>	

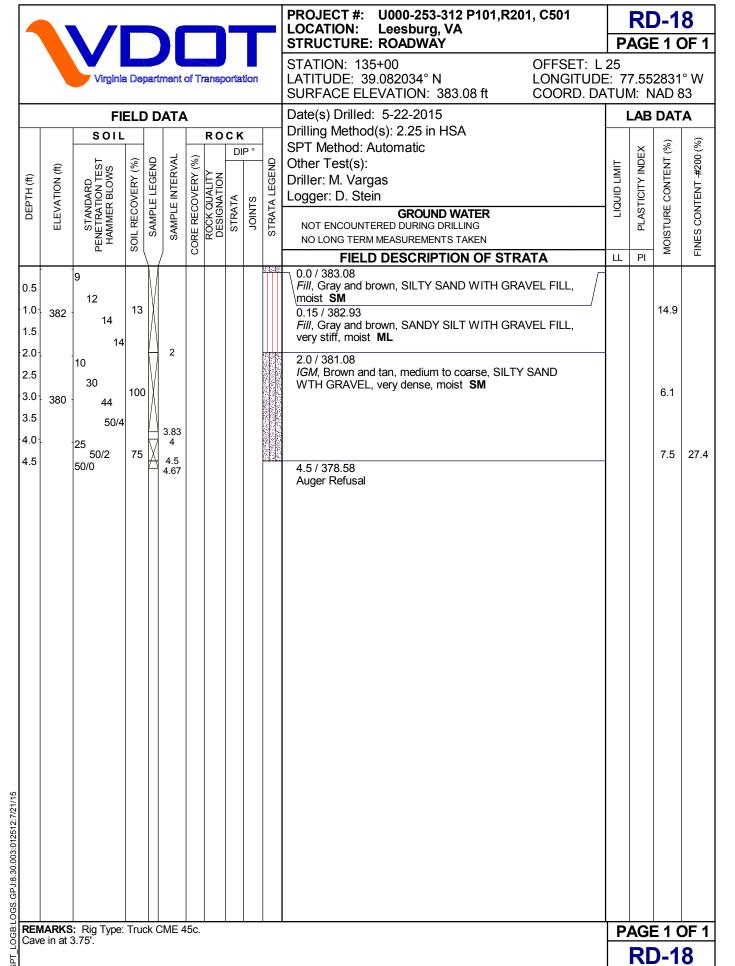


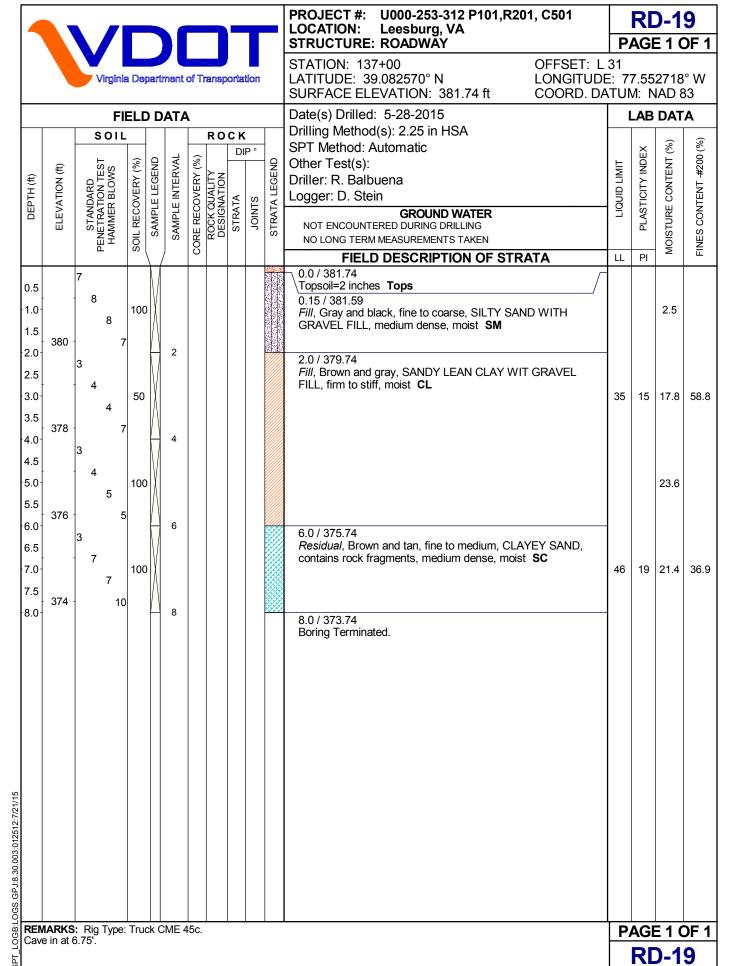
**REMARKS:** Rig Type: Truck CME 45c. Cave in at 6.75'.

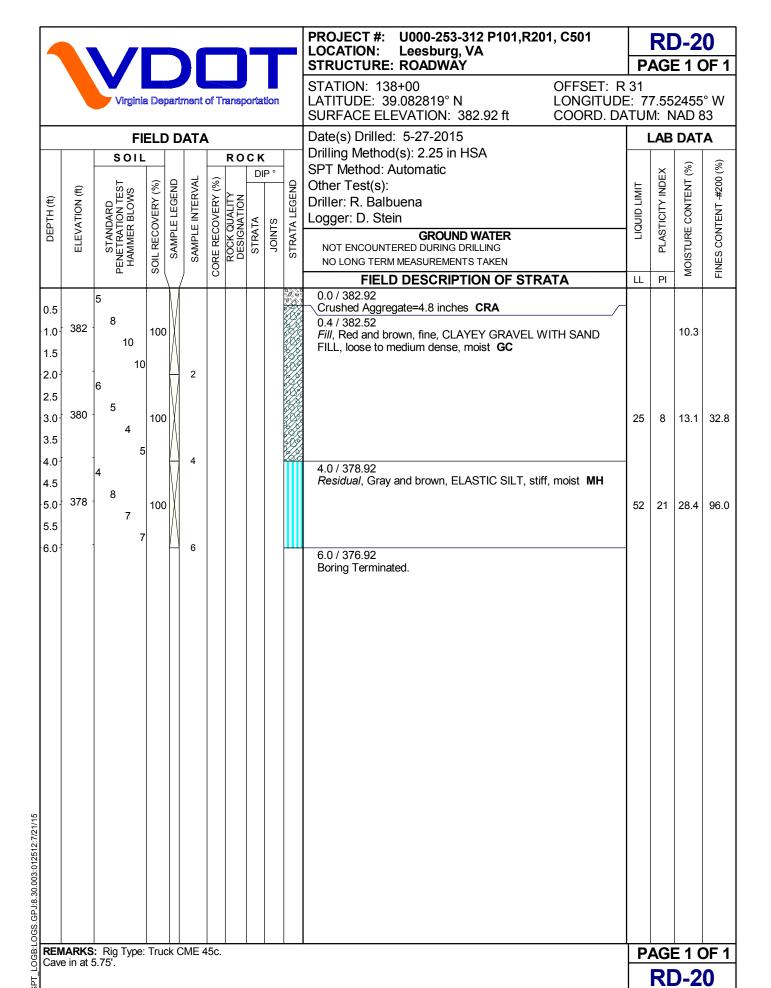
PAGE 1 OF 1 RD-15













PROJECT #: U000-253-312 P101,R201, C501

LOCATION: Leesburg, VA STRUCTURE: ROADWAY

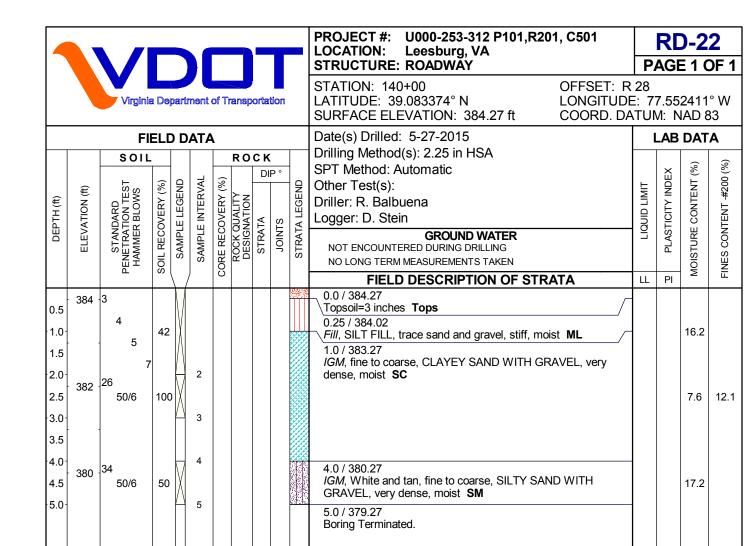
PAGE 1 OF 1

PAGE 1 OF 1 **RD-21** 

STATION: 139+00 OFFSET: L 36

LATITUDE: 39.083110° N SURFACE ELEVATION: 382.84 ft LONGITUDE: 77.552656° W COORD. DATUM: NAD 83

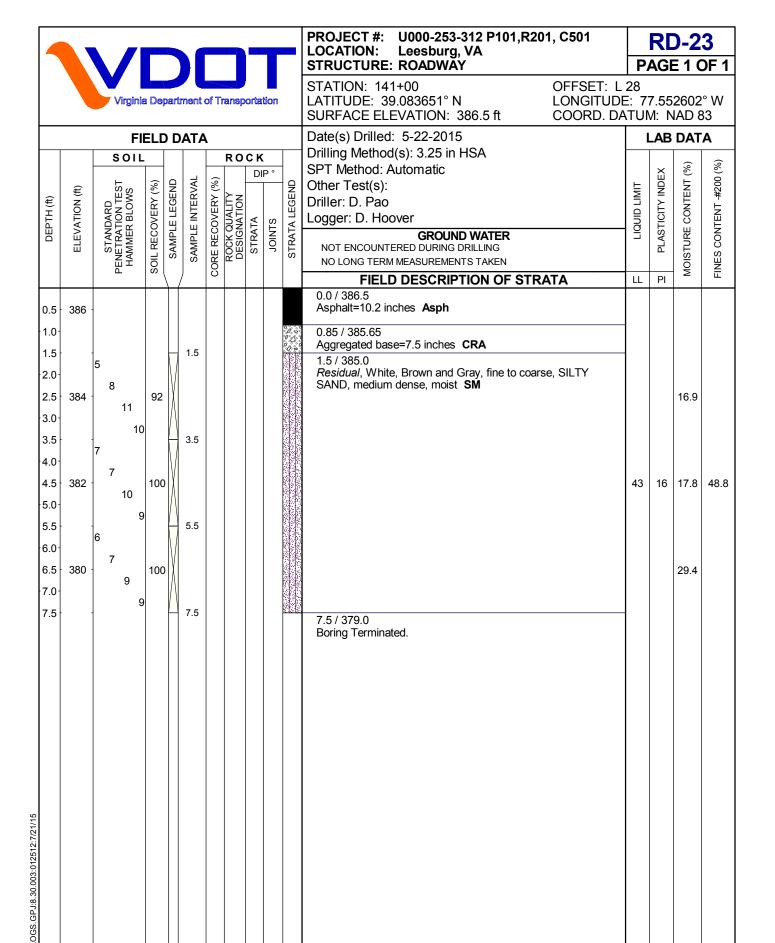
NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN  FIELD DESCRIPTION OF STRATA  LL PI		F	IELI	D D	)ATA	١				SURFACE ELEVATION: 382.84 ft COORD. DATUM Date(s) Drilled: 5-22-2015	1	B DA	
50/0 4.5 4.5 / 378.34		STANDARD PENETRATION TEST HAMMER BLOWS		SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	DI	IP°	STRATA LEGEND	SPT Method: Automatic Other Test(s): Driller: M. Vargas Logger: D. Stein  GROUND WATER  NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN  FIELD DESCRIPTION OF STRATA  0.0 / 382.84			1 WO HARMOO HOLLESTOW
	1.4 1.6 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8	9 17 50/6	5		2 2.5 4					1.5 / 381.34  IGM, Red and brown, SILT, trace sand, very hard, moist ML  4.5 / 378.34			200



OGS.GPJ:8.30.003:012512:7/21/15

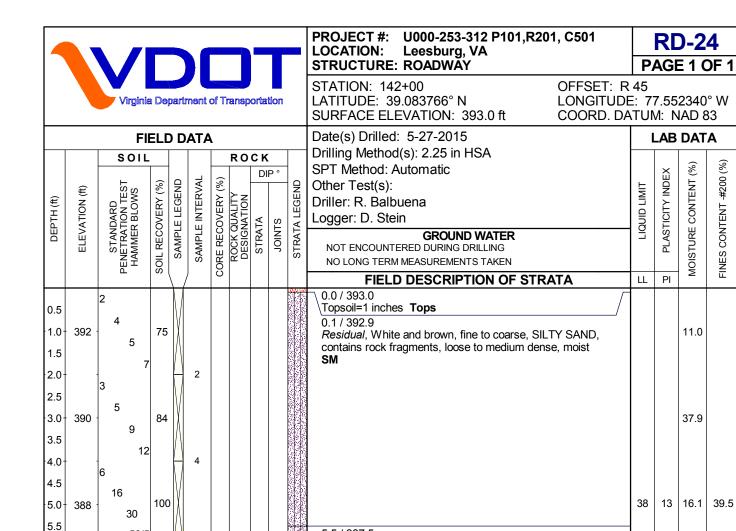
**REMARKS:** Rig Type: Truck CME 45c. Cave in at 3.5'.

PAGE 1 OF 1 RD-22



**REMARKS:** Rig Type: Truck CME 45c. Cave in at 5.75'.

PAGE 1 OF 1 RD-23



5.5 / 387.5

8.1 / 384.9 Boring Terminated.

IGM. White and brown, fine to coarse, SILTY SAND,

contains rock fragments, very dense, SM

OGS.GPJ:8.30.003:012512:7/21/15

6.0

6.5 7.0

7.5 8.0 386

**REMARKS:** Rig Type: Truck CME 45c. Offset 25' south due to slope and overhead trees. Cave in at 7.25'.

PAGE 1 OF 1

22.3

6.5

**RD-24** 

50/5

50/3

50/1

5.92

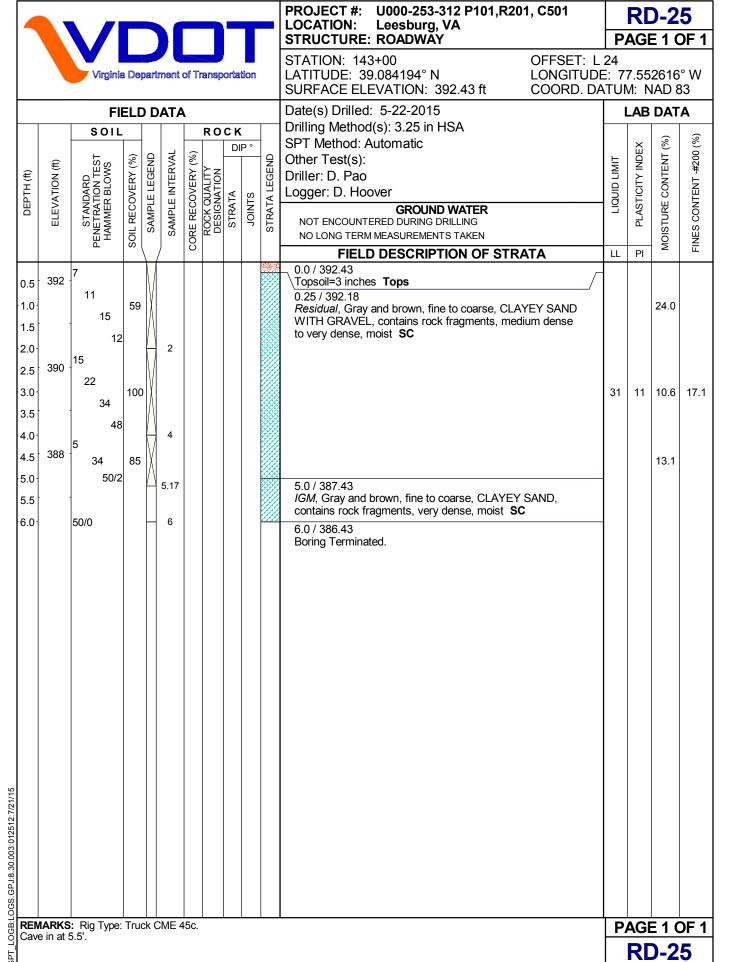
6.25

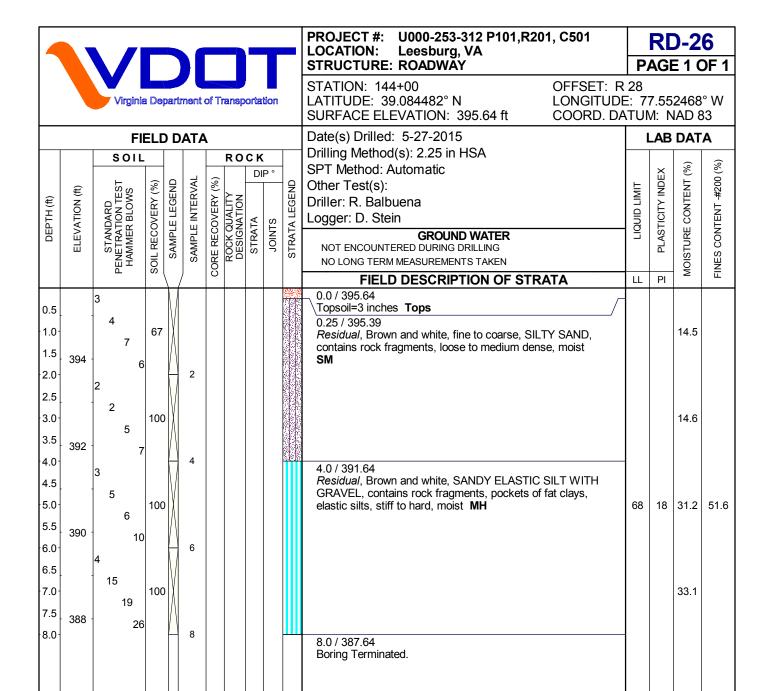
8

8.08

100 🛛

100 ≥

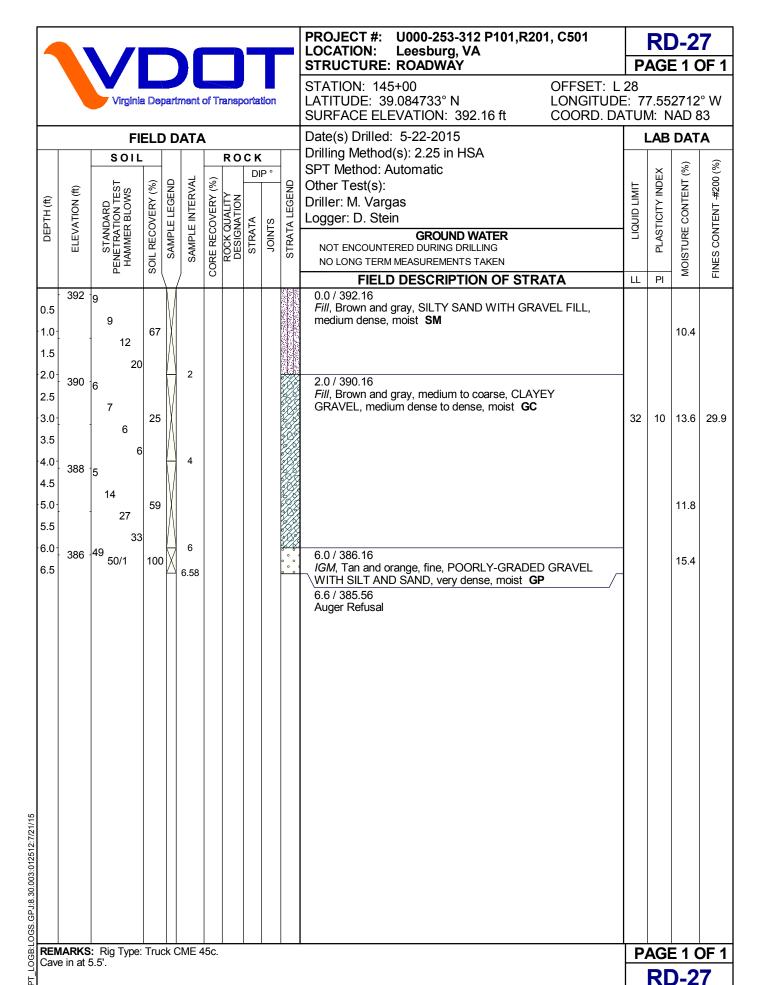




**REMARKS:** Rig Type: Truck CME 45c. Cave in at 6.25'.

PAGE 1 OF 1

.OGS.GPJ:8.30.003:012512:7/21/15





LOCATION: Leesburg, VA STRUCTURE: ROADWAY

**RD-28** 

PAGE 1 OF 1

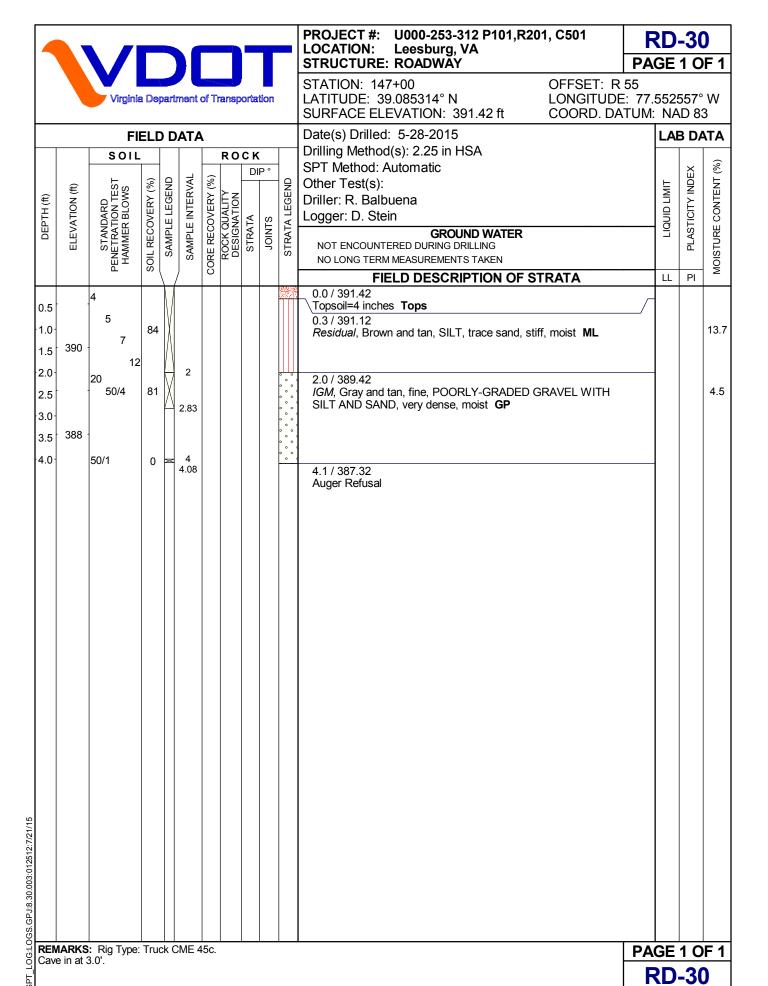
PAGE 1 OF 1 **RD-28** 

STATION: 146+00 OFFSET: R 31

LATITUDE: 39.085031° N LONGITUDE: 77.552569° W SURFACE ELEVATION: 388.84 ft COORD. DATUM: NAD 83

NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN  FIELD DESCRIPTION OF STRATA  0.0 / 388.84 Asphalt=6.6 inches Asph 0.55 / 388.29 Aggregated base=7.5 inches CRA  1.2 / 387.64 Residual, Brown and tan, fine to coarse, SILTY CLAYEY SAND, medium dense, moist SC-SM  3.2 / 385.64 IGM, Brown and tan, fine to coarse, SILTY CLAYEY SAND, contains rock fragments, very dense, moist				
SPT Method: Automatic Other Test(s): Driller: R. Balbuena Logger: D. Stein  GROUND WATER NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN  FIELD DESCRIPTION OF STRATA  1.5 1.5 1.6 1.6 1.6 1.7 1.8 1.9 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	L	LAB	DAT	Α
### FIELD DESCRIPTION OF STRATA    1.5	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	FINES CONTENT #200 (%)
1.5 388		- Di	MOIS	FINES
7.2 7.2 7.2 7.2 7.2 7.2 7.2 7.2 7.2 7.2	25	4	25.4	22.8
REMARKS: Rig Type: Truck CME 45c. Cave in at 5.25'.			E 1 (	OF 1

									PROJECT #: U000-253-312 P101,R201, C501 LOCATION: Leesburg, VA		RE	<b>)-2</b>	9
									STRUCTURE: ROADWAY			<b>E 1 C</b>	
	1	Virgini	a Depa	rtment	of Tran	sporta	<b>ition</b>		STATION: 147+00 OFFSET: L LATITUDE: 39.085267° N LONGITUDE SURFACE ELEVATION: 384.99 ft COORD. DA	E: 7			
		FI	ELD	DAT	4				Date(s) Drilled: 5-22-2015	ı	LAB	DAT	Ά
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE INTERVAL	CORE RECOVERY (%) ROCK QUALITY		( IP STNIOL	STRATA LEGEND	Drilling Method(s): 3.25 in HSA SPT Method: Automatic Other Test(s): Driller: D. Pao Logger: D. Hoover  GROUND WATER  NOT ENCOUNTERED DURING DRILLING NO LONG TERM MEASUREMENTS TAKEN	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	FINES CONTENT #200 (%)
		8 2	) S		8				FIELD DESCRIPTION OF STRATA	LL	PI	MO	E N
0.5 1.0- 1.5 2.0-	384	3 3 3	67	2					0.0 / 384.99 Topsoil=3 inches <b>Tops</b> 0.25 / 384.74 Fill, Gray and brown, LEAN CLAY WITH GRAVEL FILL, firm, moist <b>CL</b> 2.0 / 382.99			19.0	
2.5 3.0 <sup>-</sup> 3.5 4.0 <sup>-</sup> 4.5	382	5 3 2	67	4					Residual, Brown and mottled, FAT CLAY, firm, moist <b>CH</b>	68	42	29.7	89.
5.0+ 5.5 6.0+ 6.5	380	4 7 2	84	6					6.0 / 378.99  Residual, Brown and orange, SANDY SILT, stiff, moist <b>ML</b>			35.2	
7.5 3.0-	÷	7		8					8.0 / 376.99 Boring Terminated.				
<b>REM</b> Cave	ARKS	b: Rig Type 6.25'.	: Truck	CME	45c.							≣ 1 C <b>)-2</b>	





LOCATION: Leesburg, VA STRUCTURE: SWM 8-2

SWM-01A PAGE 1 OF 1

STATION: 125+00 OFFSET: R 89

LATITUDE: 39.079328° N LONGITUDE: 77.553045° W SURFACE ELEVATION: 358.64 ft COORD. DATUM: NAD 83

											SURFACE ELEVATION: 358.64 ft COORD. DA	( I U	VI. IN	IAD 8	33
		FIE	ELD	D	ATA	\					Date(s) Drilled: 5-28-2015 Drilling Method(s): 2.25 in HSA	I	_AB	DAT	Α
DEPTH (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	DI	° STNIOL	STRATA LEGEND	SPT Method: Automatic Other Test(s): Driller: R. Balbuena Logger: D. Stein  GROUND WATER  NOT ENCOUNTERED DURING DRILLING DRY AFTER 24 HRS  FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	□ PLASTICITY INDEX	MOISTURE CONTENT (%)	FINES CONTENT #200 (%)
1.0 · 1.5 · 2.0 · 2.5 · 3 · 3.0 · 4.5 · 3 · 5.5 · 6.0 · 6.0 · 6.0	358 - 356 - 3	3 14 7 12 10 10 14 16 18 50/6	75 100 67		2 4 5 6.5 8						0.0 / 358.64 Topsoil=4 inches Tops 0.3 / 358.34 Residual, Brown and black, fine to medium, SANDY LEAN CLAY, very stiff, moist CL  2.0 / 356.64 Residual, Brown and tan, SILT, very stiff, moist ML  4.5 / 354.14 IGM, Gray and tan, POORLY-GRADED SAND WITH CLAY AND GRAVEL, very dense, dry SP-SC  8.0 / 350.64 Boring Terminated.	27	9	17.6 15.0 7.0	11.6

**REMARKS:** Rig Type: Truck CME 45c. Cave in at 6.0'.



LOCATION: Leesburg, VA STRUCTURE: SWM 8-2

SWM-02A PAGE 1 OF 1

STATION: 125+50 OFFSET: R 86.5

LATITUDE: 39.079463° N LONGITUDE: 77.552999° W SURFACE ELEVATION: 356.72 ft COORD. DATUM: NAD 83

									SURFACE ELEVATION: 356.72 ft COORD. DATUM	: NA	<b>VD</b> 8	3
	FII	ELD	D	ATA	\				Date(s) Drilled: 5-30-2015 Drilling Method(s): 3.25 in HSA	LA	B DA	4ΤΑ
DEPTH (ft) ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS TI		SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION	P STNIOL	STRATA LEGEND	SPT Method: Automatic Other Test(s): Driller: R. Balbuena Logger: D. Hoover  GROUND WATER  NOT ENCOUNTERED DURING DRILLING DRY AFTER 24 HRS  FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	□ PLASTICITY INDEX	MOISTURE CONTENT (%)
0.5 1.0 1.5 2.0	1 5 9 9	50		2					0.0 / 356.72 Topsoil=4 inches Tops 0.3 / 356.42 Residual, Brown, SILT WITH SAND, contains rock fragments, stiff, moist ML			5.9
2.5 3.0 3.5 4.0	6 5 10	50		4					2.0 / 354.72  Residual, Brown, LEAN CLAY, trace sand, contains rock fragments, stiff, moist <b>CL</b>			16.3
4.5 5.0 5.5 6.0	31 50/6 50/1	42	M	5					4.0 / 352.72  IGM, Brown, LEAN CLAY, trace sand, contains rock fragments, very hard, moist CL			11.
		0		6.08					6.1 / 350.62 Auger Refusal			

**REMARKS:** Rig Type: Truck CME 45c. Cave in at 4.75'.

SWM-02A



LOCATION: Leesburg, VA STRUCTURE: Detention Facility

**SWM-03** PAGE 1 OF 1

1 . . . .

STATION: 127+00 OFFSET: R 108

LATITUDE: 39.079842° N LONGI SURFACE ELEVATION: 358.13 ft COORI

LONGITUDE: 77.552957° W COORD. DATUM: NAD 83

										SURFACE ELEVATION: 358.13 ft COORD. DATUM:	11/-	1D 0	<u>ა</u>
_	FIE	ELD	D	ATA						Date(s) Drilled: 5-30-2015 Drilling Method(s): 3.25 in HSA	LA	B DA	ATA
DEPTH (ft) 358	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	(%)	ROCK QUALITY DESIGNATION	DI	P° STNIOL	STRATA LEGEND	SPT Method: Automatic Other Test(s): Driller: R. Balbuena Logger: D. Hoover  GROUND WATER  NOT ENCOUNTERED DURING DRILLING DRY AFTER 24 HRS  FIELD DESCRIPTION OF STRATA	F LIQUID LIMIT	□ PLASTICITY INDEX	MOISTURE CONTENT (%)
0.5 1.0 1.5	50/4	46 113 100	×,	2 3.33 4 4.17 5.08						Topsoll=4 inches Tops  0.3 / 357.83  Residual, Brown, LEAN CLAY, trace sand, contains rock fragments, stiff to very hard, CL  4.0 / 354.13  IGM, Brown, LEAN CLAY, trace sand, contains rock fragments, very hard, CL  5.1 / 353.03  Auger Refusal			9.1

**REMARKS:** Rig Type: Truck CME 45c. Cave in at 4.0'.



LOCATION: Leesburg, VA STRUCTURE: Detention Facility

**SWM-04** PAGE 1 OF 1

STATION: 128+23 OFFSET: R 104

LATITUDE: 39.080173° N
SURFACE ELEVATION: 364.33 ft
LONGITUDE: 77.552921° W
COORD. DATUM: NAD 83

											SURFACE ELEVATION: 364.33 ft COORD. DATUM:	: NA	ND 8	3
		FIE	ELD	D D	ΑTΑ	<u> </u>					Date(s) Drilled: 5-30-2015	LA	B DA	AΤΑ
		SOIL					RO	СК			Drilling Method(s): 3.25 in HSA			
DЕРТН (ft)	ELEVATION (ft)	STANDARD PENETRATION TEST HAMMER BLOWS	SOIL RECOVERY (%)	SAMPLE LEGEND	SAMPLE INTERVAL	CORE RECOVERY (%)	ROCK QUALITY DESIGNATION		STNIOL	STRATA LEGEND	SPT Method: Automatic Other Test(s): Driller: R. Balbuena Logger: D. Hoover  GROUND WATER  NOT ENCOUNTERED DURING DRILLING DRY AFTER 24 HRS	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)
		Δ.	S (	\ /	<i>)</i>	ŏ					FIELD DESCRIPTION OF STRATA	LL	PI	Σ
0.5 1.0 1.5 2.0 2.5		3 3 6 6 6	50		2						0.0 / 364.33 Topsoil=4 inches <b>Tops</b> 0.3 / 364.03 Residual, Brown, LEAN CLAY, trace sand, contains rock fragments, stiff to very hard, <b>CL</b>			16.1
3.0		50/4		$\backslash \backslash$	0.00									
3.5 4.0	360	50/6	100	X	3.33						3.4 / 360.93  IGM, Brown, LEAN CLAY, trace sand, contains rock fragments, very hard, CL			4.8
4.5					4.5									
5.0 5.5 6.0	-	50/1	100	) <b>×</b>	6									
					6.08						6.1 / 358.23 Auger Refusal			

**REMARKS:** Rig Type: Truck CME 45c. Cave in at 4.25'.

# APPENDIX C LABORATORY TESTING



DMY Engineering Consultants Inc 45662 Terminal Drive, Suite 110 Virginia 20166 Telephone: (703) 665-0586

Fax: (202) 688-1918

### **SUMMARY OF LABORATORY RESULTS**

PAGE 1 OF 5

**CLIENT** Rinker Design Associates, P. C.

PROJECT NAME Sycolin Road Widening Phase IV

PROJECT NUMBER 01.02095.01

PROJECT LOCATION Leesburg. Virginia

PROJECT NUMBER	<b>R</b> 01.02095.	.01							Р	ROJECT LO	CATION Leesburg, Virginia
Sample ID	Depth (FT)	Liquid Limit	Plastic Limit	Plasticity Index	%<#200 Sieve	Water Content (%)	Proctor Method	Max Dry Density (pcf)	Optimum Moisture (%)	Oversize Fraction (%)	Sample Description/Classification
CL-01-1	0.0 - 2.0					6.0					
CL-01-2	2.0 - 4.0					20.5					
CL-01-3	4.0 - 6.0					22.6					
CL-01-4	6.0 - 8.0	56	26	30	71.5	23.9					Light brown, Fat Clay With Sand (CH)
CL-01-5	8.0 - 10.0	27	14	13	16.5	7.6					Gray, Clayey Gravel With Sand (GC)
CL-02-1	0.0 - 2.0					14.7					
CL-02-2	2.0 - 4.0					15.0					
CL-02-3	4.0 - 6.0					21.0					
CL-02-4	6.0 - 8.0	37	23	14	36.6	20.9					Brown, Clayey Gravel With Sand (GC)
CL-03-1	0.0 - 2.0					9.2					
CL-03-2	2.0 - 4.0					22.7					
CL-03-3	4.0 - 6.0					18.3					
CL-03-4	6.0 - 8.0					14.9					
CL-03-5	8.0 - 10.0					12.5					
RD-01-1	0.0 - 2.0					5.9					
RD-01-2	2.0 - 4.0	25	15	10	20.6	8.9					Red, Clayey Sand (SC)
RD-01	2.0 - 5.0	29	16	13	26.4	12.7	VTM1	130.1	10.8	No. 4-15.0 %	Red, Clayey Sand ( <b>SC</b> )
RD-01-3	4.0 - 6.0					8.5					
RD-01-4	6.0 - 8.0					15.0					
RD-02-1	0.0 - 2.0					9.9					
RD-02-2	2.0 - 4.0	25	21	4	54.3	11.2					Dark brown, Sandy Silty Clay With Gravel (CL-ML)
RD-02-3	4.0 - 6.0					7.9					
RD-02-4	6.0 - 8.0					13.5					
RD-03-1	0.0 - 2.0					4.6					
RD-03-2	2.0 - 4.0					9.4					
RD-03-3	4.0 - 6.0	27	19	8	22.7	9.5					Dark brown, Clayey Sand (SC)
RD-03-4	6.0 - 8.0					5.5					
RD-04-1	0.0 - 2.0					20.6					
RD-04-2	2.0 - 4.0	26	20	6	31.5	10.6					Reddish brown, Silty, Clayey Gravel With Sand (GC-GM)
RD-04-3	4.0 - 6.0					11.9					



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Fax: (202) 688-1918

### **SUMMARY OF LABORATORY RESULTS**

PAGE 2 OF 5

**CLIENT** Rinker Design Associates, P. C.

**PROJECT NUMBER** 01.02095.01

PROJECT NAME Sycolin Road Widening Phase IV

PROJECT LOCATION Leesburg, Virginia

PROJECT NUMBE	R 01.02095	.01							Р	ROJECT LO	CATION Leesburg, Virginia
Sample ID	Depth (FT)	Liquid Limit	Plastic Limit	Plasticity Index	%<#200 Sieve	Water Content (%)	Proctor Method	Max Dry Density (pcf)	Optimum Moisture (%)	Oversize Fraction (%)	Sample Description/Classification
RD-05-1	0.0 - 2.0					12.9					
RD-05-2	2.0 - 4.0	28	19	9	18.3	9.4					Brown, Clayey Gravel With Sand (GC)
RD-06-1	0.0 - 2.0					11.5					
RD-06	1.0 - 4.0	27	17	10	60.3	20.7	VTM1	118.8	13.0	No. 4-8.9 %	Brown, Sandy Lean Clay ( <b>CL</b> )
RD-06-2	2.0 - 4.0					11.3					
RD-06-3	4.0 - 6.0	NP	NP	NP	7.6	4.2					Gray, Poorly Graded Gravel With Silt And Sand (GP-GM)
RD-07-1	0.0 - 2.0					4.4					
RD-07-2	2.0 - 4.0					4.1					
RD-08-1	0.0 - 2.0					11.2					
RD-08-2	2.0 - 4.0	28	19	9	28.8	10.9					Brown, Clayey Gravel With Sand (GC)
RD-08-3	4.0 - 6.0					25.5					
RD-09-1	0.0 - 2.0					8.9					
RD-09-2	2.0 - 4.0	25	17	8	20.3	5.9					Brown, Clayey Gravel With Sand (GC)
RD-09-2	4.0 - 4.3					6.5					
RD-10-1	0.0 - 2.0					19.5					
RD-10-2	2.0 - 4.0					38.1					
RD-10-3	4.0 - 6.0	56	43	13	45.5	29.4					Brown, Silty Sand (SM)
RD-10-4	6.0 - 8.0					18.9					
RD-11-1	0.0 - 2.0					10.0					
RD-11-2	2.0 - 4.0	29	22	7	38.0	14.7					Brown, Silty, Clayey Sand (SC-SM)
RD-11	2.0 - 5.0	31	21	10	41.0	12.6	VTM1	135.7	8.9	No. 4-22.1 %	Brown, Clayey Sand With Gravel (SC)
RD-11-3	4.0 - 6.0					25.3					
RD-12-1	0.0 - 2.0					12.0					
RD-12-2	2.0 - 4.0					17.2					
RD-12-3	4.0 - 6.0	28	19	9	76.7	19.9					Light brown, Lean Clay With Sand (CL)
RD-12-4	6.0 - 8.0					18.6					
RD-13-1	0.0 - 2.0					10.1					
RD-13-2	2.0 - 4.0					33.9					
RD-13-3	4.0 - 6.0	NP	NP	NP	8.6	6.1					Gray, Poorly Graded Sand With Silt And Gravel (SP-SM)
RD-14-1	0.0 - 2.0					9.3					



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### **SUMMARY OF LABORATORY RESULTS**

PAGE 3 OF 5

**CLIENT** Rinker Design Associates, P. C.

PROJECT NAME Sycolin Road Widening Phase IV

PROJECT NUMBER 01.02095.01 PROJECT LOCATION Leesburg, Virginia

	01.02095.	• •							•	NOOLO I LO	CATION Leesburg, Virginia
Sample ID	Depth (FT)	Liquid Limit	Plastic Limit	Plasticity Index	%<#200 Sieve	Water Content (%)	Proctor Method	Max Dry Density (pcf)	Optimum Moisture (%)	Oversize Fraction (%)	Sample Description/Classification
RD-14-2	2.0 - 4.0	28	19	9	22.5	9.4					Brown, Clayey Sand (SC)
RD-14-3	4.0 - 6.0					5.0					
RD-14-4	6.0 - 8.0					5.9					
RD-15-1	0.0 - 2.0					8.9					
RD-15-2	2.0 - 4.0					26.9					
RD-15-3	4.0 - 6.0	34	26	8	45.3	21.9					Brown, Silty Sand ( <b>SM</b> )
RD-15-4	6.0 - 8.0					17.1					
RD-16-1	0.0 - 2.0					7.7					
RD-16-2	2.0 - 2.3					8.8					
RD-17-1	0.0 - 2.0					9.8					
RD-17-2	2.0 - 2.4					6.3					
RD-18-1	0.0 - 2.0					14.9					
RD-18-2	2.0 - 4.0					6.1					
RD-18-3	4.0 - 6.0	NP	NP	NP	27.4	7.5					Tan, Silty Sand With Gravel ( <b>SM</b> )
RD-19-1	0.0 - 2.0					2.5					
RD-19-2	2.0 - 4.0	35	20	15	58.8	17.8					Sandy Lean Clay With Gravel (CL)
RD-19-3	4.0 - 6.0					23.6					
RD-19-4	6.0 - 8.0	46	27	19	36.9	21.4					Light brown, Clayey Sand ( <b>SC</b> )
RD-20-1	0.0 - 2.0					10.3					
RD-20-2	2.0 - 4.0	25	17	8	32.8	13.1					Gray, Clayey Gravel With Sand (GC)
RD-20-3	4.0 - 6.0	52	31	21	96.0	28.4					Brown, Elastic Silt ( <b>MH</b> )
RD-21-1	0.0 - 2.0					20.5					
RD-22-1	0.0 - 2.0					16.2					
RD-22	1.0 - 4.0	33	18	15	43.3	7.5	VTM1	130.9	9.4	No. 4-21.6 %	Brown, Clayey Sand With Gravel (SC)
RD-22-2	2.0 - 4.0	NP	NP	NP	12.1	7.6					Gray, Silty Sand With Gravel (SM)
RD-22-3	4.0 - 6.0					17.2					
RD-22-3 RD-23-1	1.5 - 3.5					16.9					
RD-23-2	3.5 - 5.5	43	27	16	48.8	17.8					Gray, Silty Sand (SM)
RD-23-2 RD-23-3	5.5 - 7.5					29.4					
RD-24-1	0.0 - 2.0					11.0					



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### **SUMMARY OF LABORATORY RESULTS**

PAGE 4 OF 5

**CLIENT** Rinker Design Associates, P. C.

PROJECT NAME Sycolin Road Widening Phase IV

PROJECT NUMBER 01.02095.01

PROJECT LOCATION Leesburg, Virginia

PROJECT NUMBER	<b>1</b> 01.02095	.01						PROJECT LOCATION Leesburg, Virginia					
Sample ID	Depth (FT)	Liquid Limit	Plastic Limit	Plasticity Index	%<#200 Sieve	Water Content (%)	Proctor Method	Max Dry Density (pcf)	Optimum Moisture (%)	Oversize Fraction (%)	Sample Description/Classification		
RD-24-2	2.0 - 4.0					37.9							
RD-24-3	4.0 - 6.0	38	25	13	39.5	16.1					Gray, Silty Sand (SM)		
RD-24-4	6.0 - 8.0					22.3							
RD-24-5	8.0 - 10.0					6.5							
RD-25-1	0.0 - 2.0					24.0							
RD-25-2	2.0 - 4.0	31	20	11	17.1	10.6					Brown, Clayey Sand With Gravel (SC)		
RD-25-3	4.0 - 5.2					13.1							
RD-26-1	0.0 - 2.0					14.5							
RD-26-2	2.0 - 4.0					14.6							
RD-26-3	4.0 - 6.0	68	50	18	51.6	31.2					Gray, Sandy Elastic Silt With Gravel (MH)		
RD-26-4	6.0 - 8.0					33.1							
RD-27-1	0.0 - 2.0					10.4							
RD-27-2	2.0 - 4.0	32	22	10	29.9	13.6					Brown, Clayey Gravel ( <b>GC</b> )		
RD-27	2.0 - 5.0	28	18	10	44.1	7.4	VTM1	127.7	10.4	No. 4-24.0 %	Brown, Clayey Sand With Gravel (SC)		
RD-27-3	4.0 - 6.0					11.8							
RD-27-4	6.0 - 8.0					15.4							
RD-28-1	0.0 - 2.0					25.4							
RD-28-2	2.0 - 4.0	25	21	4	22.8	6.2					Brown, Silty, Clayey Sand ( <b>SC-SM</b> )		
RD-28-3	4.0 - 6.0					16.1							
RD-29-1	0.0 - 2.0					19.0							
RD-29-2	2.0 - 4.0	68	26	42	89.6	29.7					Light brown, Fat Clay ( <b>CH</b> )		
RD-29-3	4.0 - 6.0					35.2							
RD-29-4	6.0 - 8.0					19.7							
RD-30-1	0.0 - 2.0					13.7							
RD-30-2	2.0 - 4.0					4.5							
SWM-01A-1	0.0 - 2.0					17.6							
SWM-01A-2	2.0 - 4.0					15.0							
SWM-01A-3	4.0 - 6.0	27	18	9	11.6	7.0					Brown, Poorly Graded Sand With Clay And Gravel (SP-SC)		
SWM-01A-4	6.0 - 6.5					2.3							
SWM-02A-1	0.0 - 2.0					5.9							



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### **SUMMARY OF LABORATORY RESULTS**

PAGE 5 OF 5

Fax: (202) 688-1918 **CLIENT** Rinker Design Associates, P. C.

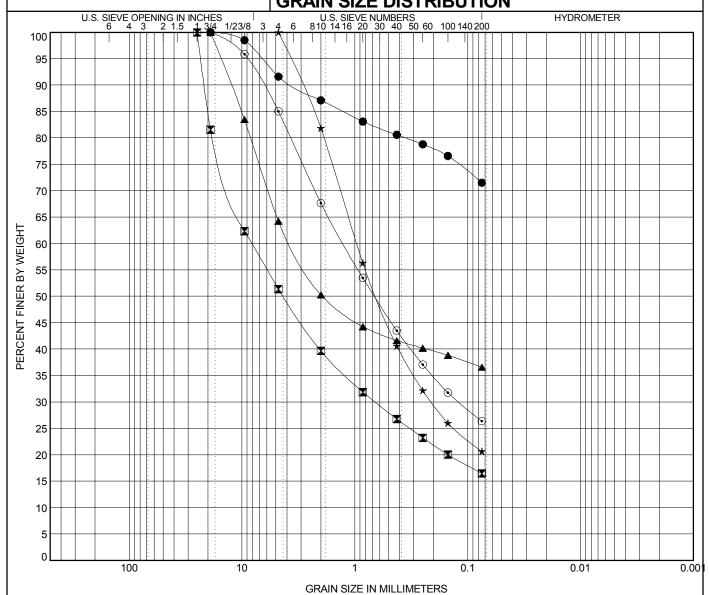
PROJECT NAME Sycolin Road Widening Phase IV

PROJECT NUMBER	<b>R</b> 01.02095.	.01					PROJECT LOCATION Leesburg, Virginia					
Sample ID	Depth (FT)	Liquid Limit	Plastic Limit	Plasticity Index	%<#200 Sieve	Water Content (%)	Proctor Method	Max Dry Density (pcf)	Optimum Moisture (%)	Oversize Fraction (%)	Sample Description/Classification	
SWM-02A-2	2.0 - 4.0					16.3						
SWM-02A-3	4.0 - 6.0					11.4						
SWM-03-1	0.0 - 2.0					13.8						
SWM-03-2	2.0 - 4.0					9.1						
SWM-03-3	4.0 - 4.2					6.4						
SWM-04-1	0.0 - 2.0					16.1						
SWM-04-2	2.0 - 4.0					11.3						
SWM-04-3	4.0 - 4.5					4.8						



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



CORRI ES	GRA	VEL		SAND		SILT OR CLAY
CODDLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

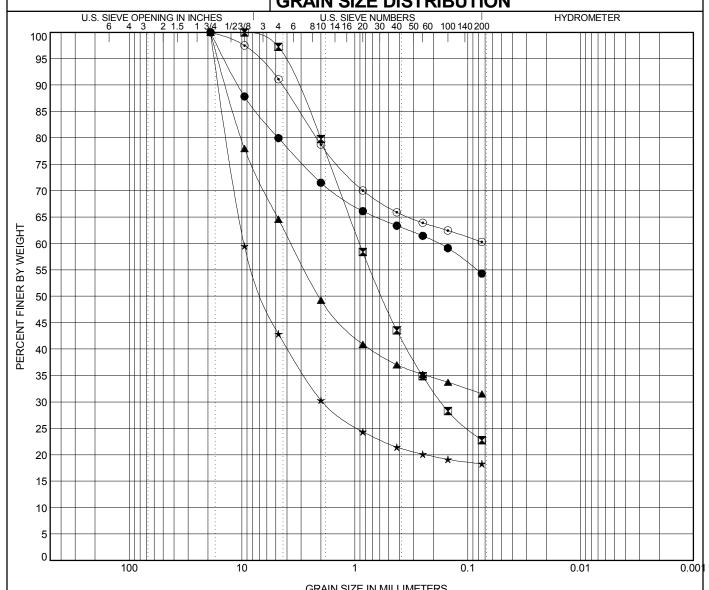
S	SAMPLE DEPTH (FT) SAMPLE DESCRIPTION										PI
•	CL-01-4	6.0-8.0		Light brown, Fat Clay With Sand (CH)							
X	CL-01-5	8.0-10.0		Gray, Clayey Gravel With Sand (GC)							
▲	CL-02-4	6.0-8.0		Brown, Clayey Gravel With Sand (GC)							14
*	RD-01-2	2.0-4.0		Re	d, Clayey S	and (SC)			25	15	10
•	RD-01	2.0-5.0		Red, Clayey Sand (SC)						16	13
			D100 D60 D30 D10 %Gravel %						and	%Silt	

			D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	CL-0	1 6.0-8.0	19				8.4	20.1	71	1.5
6/29/15	CL-0	1 8.0-10.0	25	8.198	0.66		48.6	34.9	16	6.5
		2 6.0-8.0	19	3.652			35.8	27.7	36	6.6
NSIZ	RD-0	1 2.0-4.0	4.75	0.961	0.209		0.0	79.4	20	).6
GRAIN SIZE	RD-0	1 2.0-5.0	19	1.26	0.12		15.0	58.7	26	6.4



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



### GRAIN SIZE IN MILLIMETERS

CORRI ES	GRA	VEL		SAND	1	SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

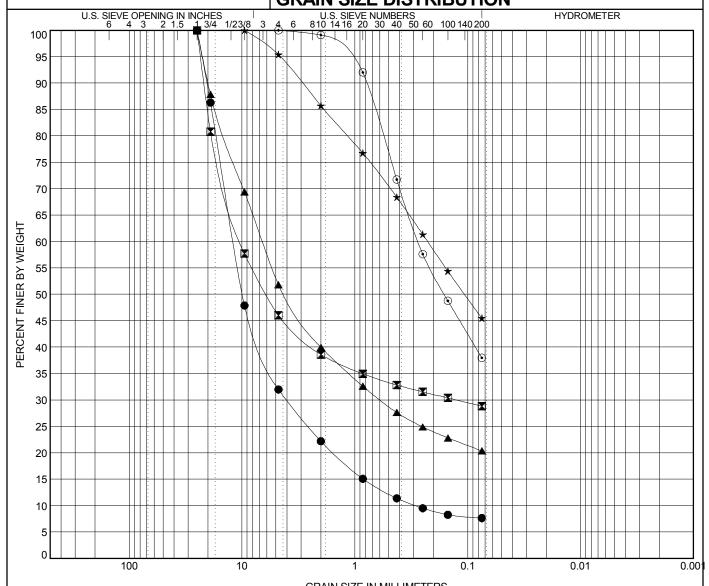
S	SAMPLE	DEPTH (FT)		SAM		LL	PL	PI				
•	RD-02-2	2.0-4.0	Dark	Dark brown, Sandy Silty Clay With Gravel (CL-ML)							4	
X	RD-03-3	4.0-6.0		Dark brown, Clayey Sand ( <b>SC</b> )							8	
▲	RD-04-2	2.0-4.0	Reddish	Reddish brown, Silty, Clayey Gravel With Sand (GC-GM)							6	
*	RD-05-2	2.0-4.0		Brown, Clayey Gravel With Sand (GC)						19	9	
•	RD-06	1.0-4.0		Brown, Sandy Lean Clay ( <b>CL</b> )						17	10	
			D100	D60	D30	D10	%Gravel	%Sa	and	%Silt		%0

				5.00			2.0	700.000	,00aa	700 iii
	•	RD-02	2.0-4.0	19	0.182			20.0	25.7	54.3
29/15		RD-03	4.0-6.0	9.5	0.907	0.172		2.7	74.5	22.7
Œ 6/;	•	RD-04	2.0-4.0	19	3.662			35.4	33.1	31.5
N SI	*	RD-05	2.0-4.0	19	9.579	1.914		57.1	24.6	18.3
GRAI	•	RD-03 RD-04 RD-05 RD-06	1.0-4.0	19				8.9	30.8	60.3



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



### GRAIN SIZE IN MILLIMETERS

CORRIES	GRA	VEL		SAND		SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

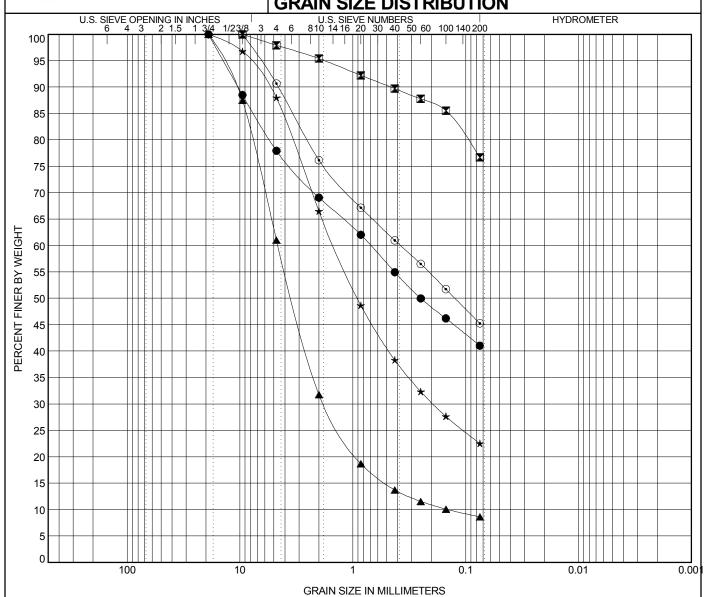
[5	SAMPLE	DEPTH (FT)	SAMPLE DESCRIPTION	LL	PL	PI
•	RD-06-3	4.0-6.0	Gray, Poorly Graded Gravel With Silt And Sand (GP-GM)	NP	NP	NP
X	RD-08-2	2.0-4.0	Brown, Clayey Gravel With Sand (GC)	28	19	9
	RD-09-2	2.0-4.0	Brown, Clayey Gravel With Sand (GC)	25	17	8
*	RD-10-3	4.0-6.0	Brown, Silty Sand ( <b>SM</b> )	56	43	13
0	RD-11-2	2.0-4.0	Brown, Silty, Clayey Sand (SC-SM)	29	22	7

			D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	PD-06-3	4.0-6.0	25	11.822	3.99	0.289	68.0	24.4	7.	.6
GRAIN SIZE 6/29/15	RD-08-2	2.0-4.0	25	10.176	0.127		54.0	17.2	28	3.8
Æ 6/	RD-09-2	2.0-4.0	25	6.56	0.593		48.2	31.5	20	).3
NSIZ	★ RD-10-3	4.0-6.0	9.5	0.226			4.6	49.9	45	5.5
GRAI	P RD-11-2	2.0-4.0	4.75	0.273			0.0	62.0	38	3.0



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



CORRLES	GRAVEL			SAND		SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

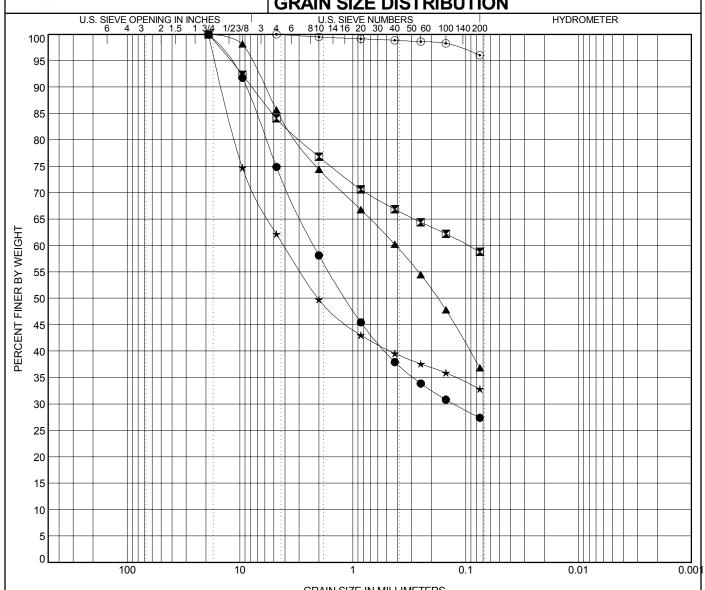
[5	SAMPLE	DEPTH (FT)	SAMPLE DESCRIPTION	LL	PL	PI
•	RD-11	2.0-5.0	Brown, Clayey Sand With Gravel (SC)	31	21	10
X	RD-12-3	4.0-6.0	Light brown, Lean Clay With Sand ( <b>CL</b> )	28	19	9
	RD-13-3	4.0-6.0	Gray, Poorly Graded Sand With Silt And Gravel (SP-SM)	NP	NP	NP
*	RD-14-2	2.0-4.0	Brown, Clayey Sand ( <b>SC</b> )	28	19	9
0	RD-15-3	4.0-6.0	Brown, Silty Sand ( <b>SM</b> )	34	26	8

			D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	RD-11-	2.0-5.0	19	0.697			22.1	36.9	41	.0
29/15	RD-12-3	4.0-6.0	9.5				2.1	21.2	76	5.7
GRAIN SIZE 6/29/15	RD-13-3	4.0-6.0	19	4.61	1.783	0.145	39.0	52.4	8	.6
N SIZ	RD-14-2	2.0-4.0	19	1.464	0.194		12.0	65.5	22	2.5
GRA	RD-15-3	4.0-6.0	9.5	0.379			9.3	45.4	45	5.3



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



### GRAIN SIZE IN MILLIMETERS

CORRI ES	GRA	VEL		SAND	1	SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

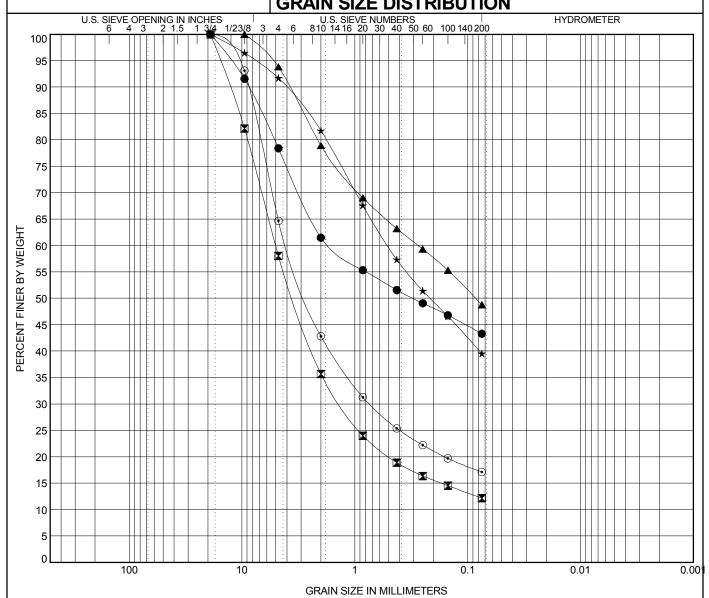
S	AMPLE	DEPTH (FT)		SAI	MPLE DESC	CRIPTION			LL	PL	PI	l
•	RD-18-3	4.0-6.0		Tan, Sil	ty Sand Wit	h Gravel ( <b>S</b>	M)		NP	NP	NF	>
X	RD-19-2	2.0-4.0		Sandy L	ean Clay W	ith Gravel (	CL)		35	20	15	5
▲	RD-19-4	6.0-8.0		Light b	rown, Claye	ey Sand ( <b>S</b> 0	<b>S</b> )		46	27	19	•
*	RD-20-2	2.0-4.0		Gray, Cla	yey Gravel	With Sand	(GC)		25	17	8	
•	RD-20-3 4.0-6.0 Brown, Elastic Silt (MH)					52	31	21	ī			
	D100 D60 D30 D10 %Gravel				%Sa	and	%Silt					

				D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
		RD-18-3	4.0-6.0	19	2.203	0.128		25.1	47.5	27	<b>7.4</b>
29/15		RD-19-2	2.0-4.0	19	0.095			15.9	25.3	58	3.8
,9 4	<b>A</b>	RD-19-4	6.0-8.0	19	0.416			14.3	48.8	36	6.9
NSIZ	*	RD-20-2	2.0-4.0	19	4.084			37.8	29.4	32	2.8
GRAIN SIZE 6/29/15	<b>9</b>	RD-20-3	4.0-6.0	4.75				0.0	4.0	96	6.0



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



CORRI ES	GRA	VEL	SAND			SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAY

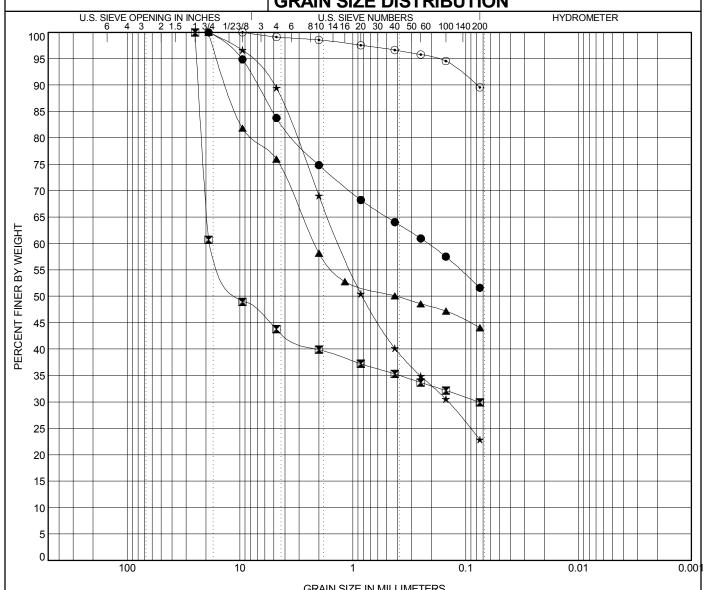
5	SAMPLE	DEPTH (FT)	SAMPLE DESCRIPTION	LL	PL	PI
•	RD-22	1.0-4.0	Brown, Clayey Sand With Gravel (SC)	33	18	15
	RD-22-2	2.0-4.0	Gray, Silty Sand With Gravel (SM)	NP	NP	NP
	RD-23-2	3.5-5.5	Gray, Silty Sand ( <b>SM</b> )	43	27	16
*	RD-24-3	4.0-6.0	Gray, Silty Sand ( <b>SM</b> )	38	25	13
0	RD-25-2	2.0-4.0	Brown, Clayey Sand With Gravel (SC)	31	20	11

				D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	•	RD-22-	1.0-4.0	19	1.629			21.6	35.1	43	3.3
29/15		RD-22-2	2.0-4.0	19	5.026	1.319		42.0	45.9	12	2.1
Œ 6/;	•	RD-23-2	3.5-5.5	9.5	0.274			6.1	45.1	48	3.8
NSIZ	*	RD-24-3	4.0-6.0	19	0.509			8.3	52.2	39	).5
GRAIN SIZE 6/29/15	•	RD-25-2	2.0-4.0	19	3.95	0.733		35.3	47.6	17	'.1



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

# **GRAIN SIZE DISTRIBUTION**



### GRAIN SIZE IN MILLIMETERS

COPPLES	GRA	VEL		SAND		SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

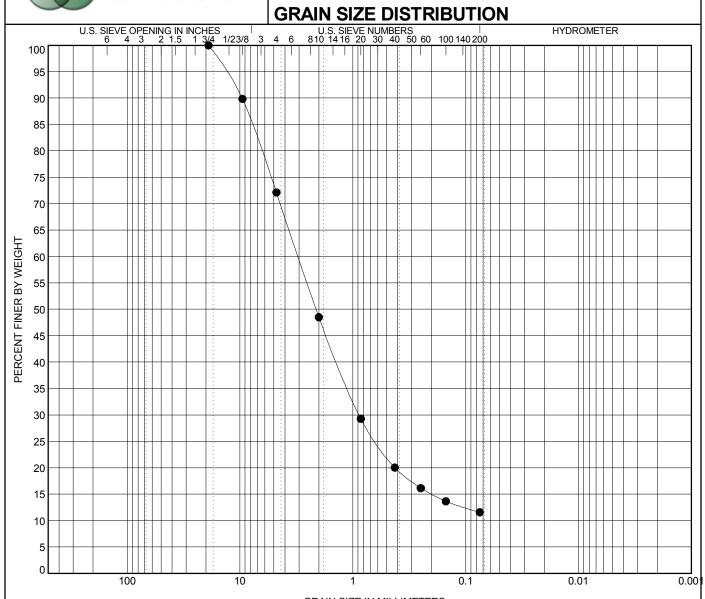
S	AMPLE	DEPTH (FT)		SAI	MPLE DESC	CRIPTION			LL	PL	PI	l
•	RD-26-3	4.0-6.0		Gray, Sandy	y Elastic Silt	With Grave	el (MH)		68	50	18	3
X	RD-27-2	2.0-4.0		Brow	n, Clayey G	Gravel (GC)			32	22	10	)
▲	RD-27	2.0-5.0		Brown, Cla	ayey Sand V	Vith Gravel	(SC)		28	18	10	)
*	RD-28-2	2.0-4.0		Brown, S	Silty, Clayey	Sand (SC-S	SM)		25	21	4	
•	<b>RD-29-2 2.0-4.0</b> Light brown, Fat Clay ( <b>CH</b> )					68	26	42	2			
	D100 D60 D30 D10 %Gravel %S					%Sa	and	%Silt				

			D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	RD-26-3	4.0-6.0	19	0.217			16.2	32.2	51	.6
29/15	RD-27-2	2.0-4.0	25	18.243	0.077		56.2	13.8	29	).9
/9 II	RD-27-	2.0-5.0	19	2.186			24.0	32.0	44	l.1
NSIZ	RD-28-2	2.0-4.0	19	1.319	0.143		10.5	66.6	22	2.8
GRAIN SIZE 6/29/15	RD-29-2	2.0-4.0	9.5				0.9	9.5	89	).6



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.



### GRAIN SIZE IN MILLIMETERS

CORRI ES	GRA	VEL		SAND		SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

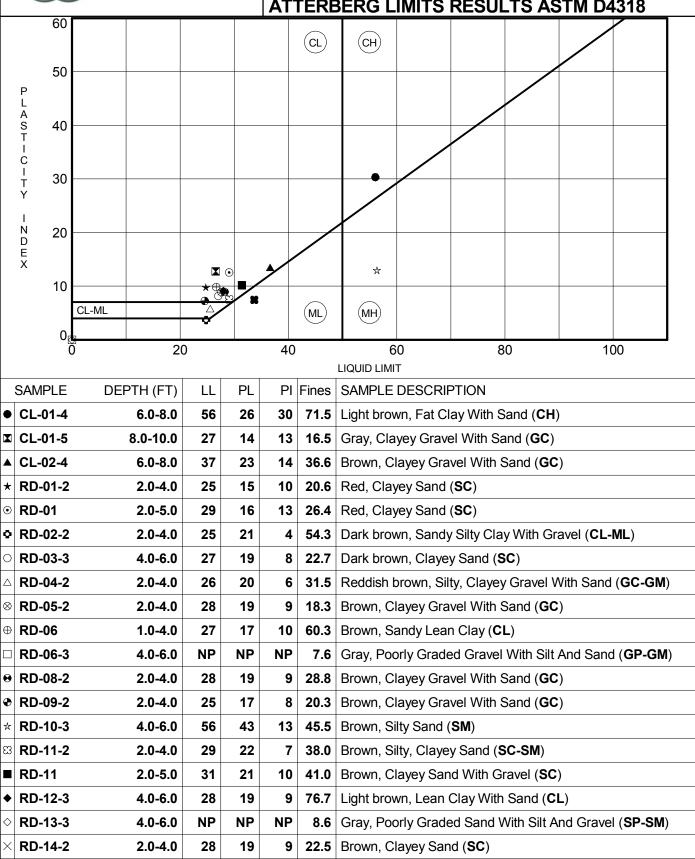
(J)	AMPLE	DEPTH (FT)	SAMPLE DESCRIPTION	LL	PL	PI
•	SWM-0	1A-3 4.0-6.0	Brown, Poorly Graded Sand With Clay And Gravel (SP-SC)	27	18	9

		D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
	SWM-01A-3 4.0-6.0	19	3.047	0.878		27.9	60.6	11	.6
29/15									
9									
N SIZE									
GRAIN									



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

### ATTERBERG LIMITS RESULTS ASTM D4318



RD-15-3

4.0-6.0

34

26

8

45.3

Brown, Silty Sand (SM)



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

### ATTERBERG LIMITS RESULTS ASTM D4318 (CL) (cH) 50 LASTICITY $^{\mathfrak{Z}}$ 40 30 ı N D 20 E X • 10 CL-ML (ML) (MH) 20 40 60 80 100 LIQUID LIMIT **SAMPLE** DEPTH (FT) LL PLPI Fines | SAMPLE DESCRIPTION ● RD-18-3 4.0-6.0 NP NP NP **27.4** Tan, Silty Sand With Gravel (**SM**) **▼** RD-19-2 2.0-4.0 20 15 58.8 Sandy Lean Clay With Gravel (CL) 35 **RD-19-4** 27 **36.9** Light brown, Clayey Sand (**SC**) 6.0-8.0 46 19 **RD-20-2** 2.0-4.0 25 17 8 32.8 Gray, Clayey Gravel With Sand (GC) ⊙ RD-20-3 4.0-6.0 52 31 21 96.0 Brown, Elastic Silt (MH) 18 43.3 Brown, Clayey Sand With Gravel (SC) 1.0-4.0 33 15 ol **RD-22-2** 2.0-4.0 NP NP NP 12.1 Gray, Silty Sand With Gravel (SM) Δ RD-23-2 3.5-5.5 43 27 16 48.8 Gray, Silty Sand (SM) RD-24-3 4.0-6.0 25 **39.5** Gray, Silty Sand (**SM**) $\otimes$ 38 13 ⊕ RD-25-2 20 Brown, Clayey Sand With Gravel (SC) 2.0-4.0 31 11 17.1 □ RD-26-3 4.0-6.0 50 **51.6** Gray, Sandy Elastic Silt With Gravel (**MH**) 68 18 RD-27-2 2.0-4.0 22 10 29.9 Brown, Clayey Gravel (GC) **⊕** RD-27 2.0-5.0 18 10 Brown, Clayey Sand With Gravel (SC) 28 44.1 ☆ **RD-28-2** 2.0-4.0 25 21 4 22.8 Brown, Silty, Clayey Sand (SC-SM) **RD-29-2** 2.0-4.0 26 42 89.6 Light brown, Fat Clay (CH) 68 11.6 Brown, Poorly Graded Sand With Clay And Gravel (SP-SC) ■ SWM-01A-3 4.0-6.0 27 18 9

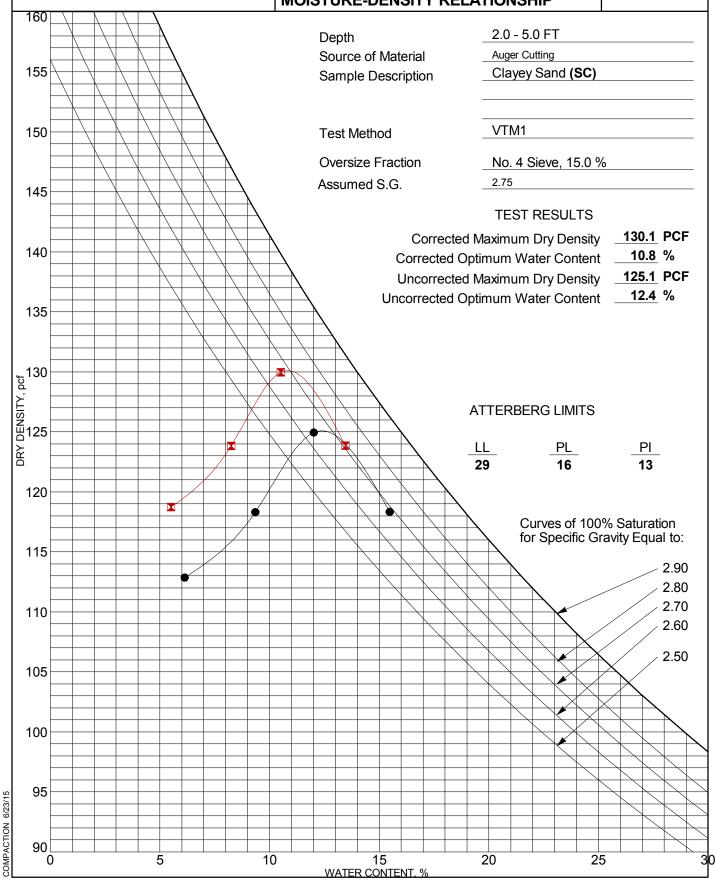
ATTERBERG LIMITS 6/29/15



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

**RD-01** 

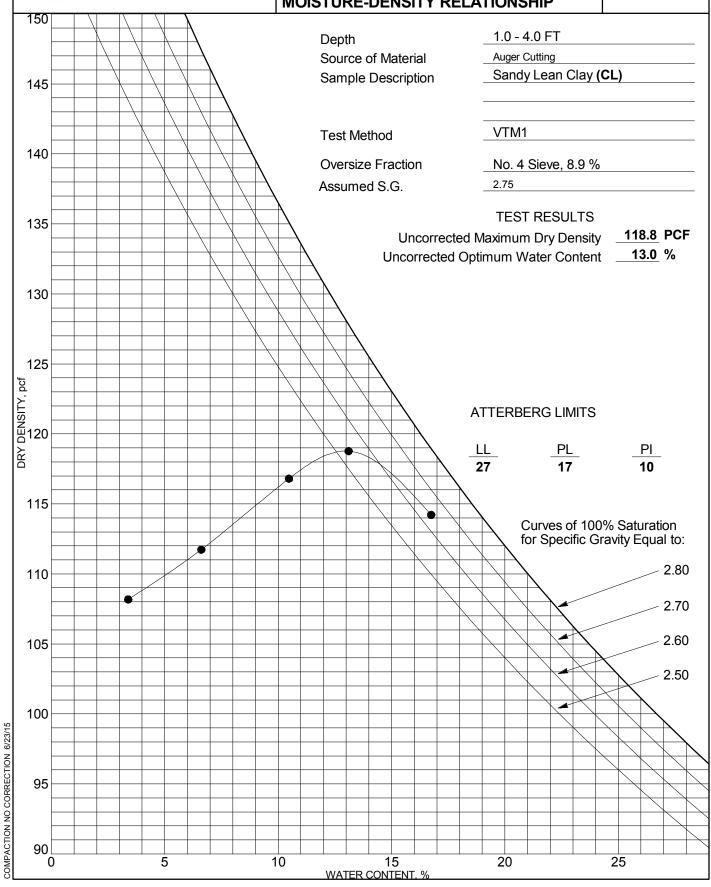




PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

**RD-06** 

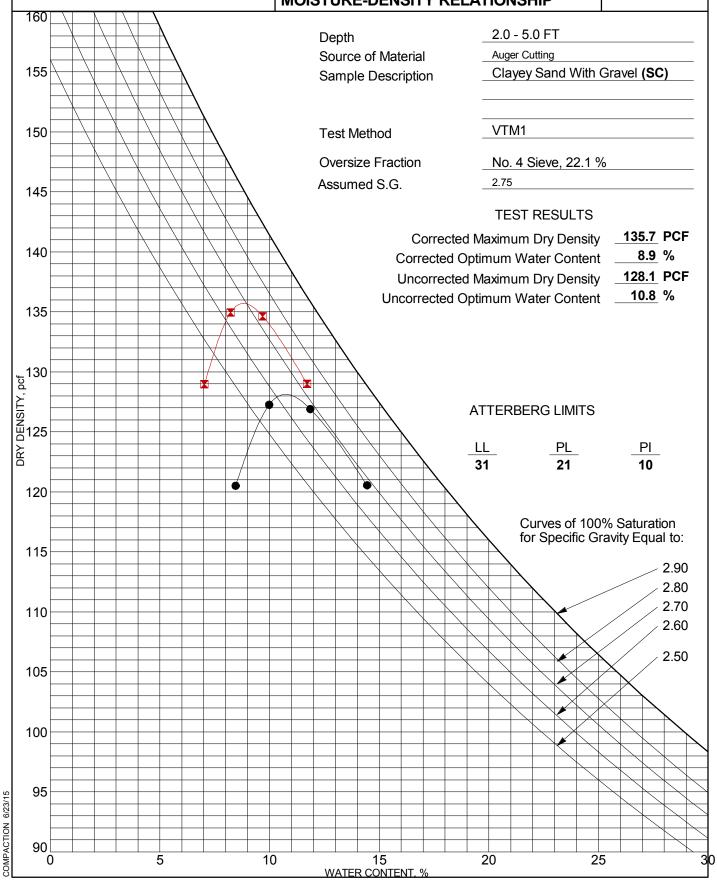




PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

**RD-11** 

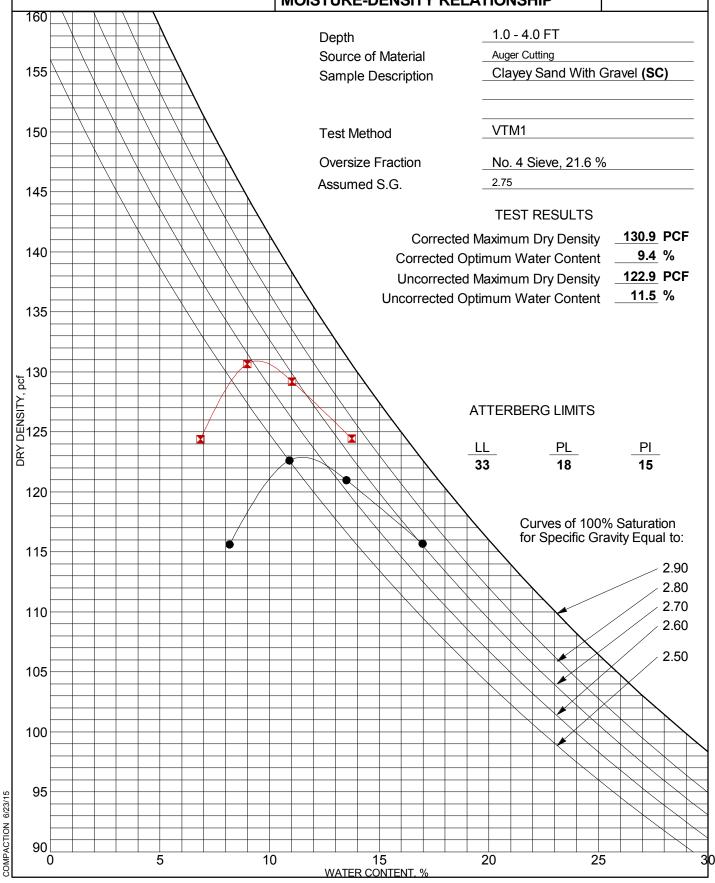




PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

**RD-22** 

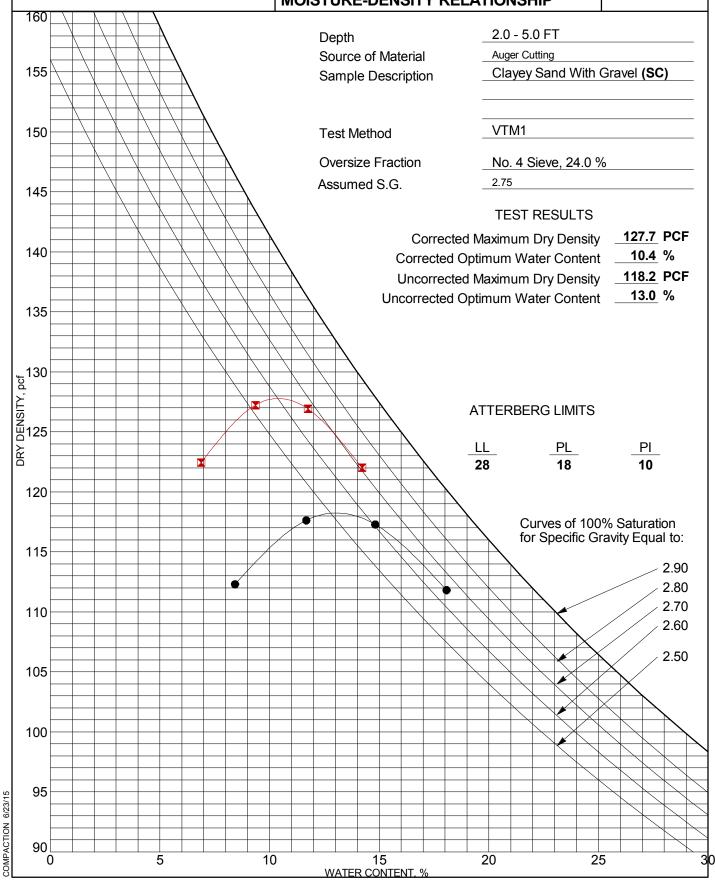




PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

**RD-27** 





PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

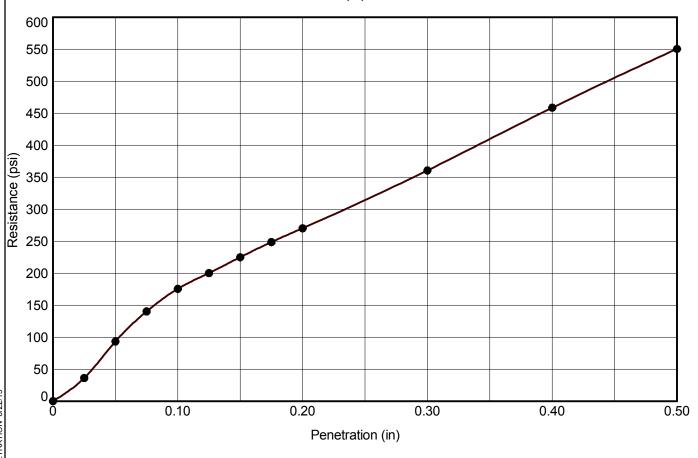
### **CALIFORNIA BEARING RATIO**

SAMPLE ID RD-01
SAMPLE DEPTH 2.0 - 5.0 FT
SOURCE MATERIAL Auger Cutting
SAMPLE DESCRIPTION Clayey Sand (SC)

Condition	Saturated	CBR Method	VTM 8
Surcharge Wt., (lb)	25	Proctor Method	VTM1
Initial Dry Density, pcf	128.7	Maximum Dry Density, pcf	130.1
Initial Moisture, %	9.7	Optimum Water Content, %	10.8
Initial Compaction, %	98.9	Percent Oversize, % No. 4 Sieve	15.0
CBR at 0.1 inch Penetration, %	18.5	Gravel, %	15.0
CBR at 0.2 inch Penetration, %	19.0	Fines, %	26.4
Swell, %	0.5	Liquid Limit	29
Top 1" Moisture After Penetration, %	13.3	Plastic Limit	16
Moisture After Penetration, %	11.7	Plasticity Index	13

Remarks:

### Penetration (in) Vs. Resistance



CBR PENETRATION 6/22/15

Prepared By:

Reviewed By:\_



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

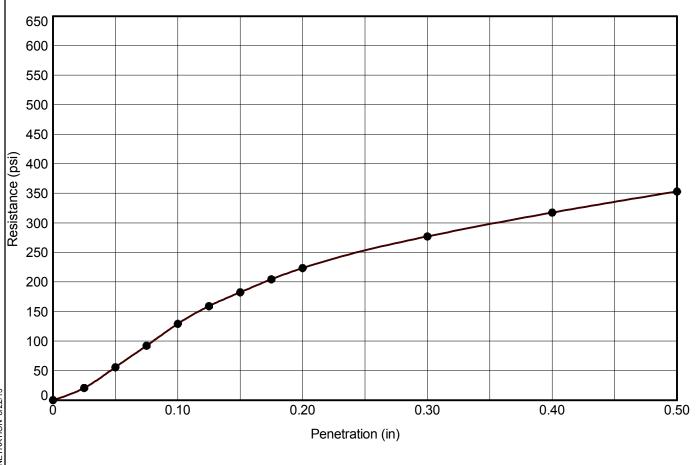
### **CALIFORNIA BEARING RATIO**

SAMPLE ID RD-06
SAMPLE DEPTH 1.0 - 4.0 FT
SOURCE MATERIAL Auger Cutting
SAMPLE DESCRIPTION Sandy Lean Clay (CL)

Condition	Saturated	CBR Method	VTM 8
Surcharge Wt., (lb)	25	Proctor Method	VTM1
Initial Dry Density, pcf	120.6	Maximum Dry Density, pcf	118.8
Initial Moisture, %	11.5	Optimum Water Content, %	13.0
Initial Compaction, %	101.5	Percent Oversize, % No. 4 Sieve	8.9
CBR at 0.1 inch Penetration, %	14.5	Gravel, %	8.9
CBR at 0.2 inch Penetration, %	15.3	Fines, %	60.3
Swell, %	1.1	Liquid Limit	27
Top 1" Moisture After Penetration, %	15.3	Plastic Limit	17
Moisture After Penetration, %	12.5	Plasticity Index	10

Remarks:

### Penetration (in) Vs. Resistance



**CBR PENETRATION 6/22/15** 

Prepared By:

Reviewed By:\_\_



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia

CLIENT: Rinker Design Associates, P. C.

### **CALIFORNIA BEARING RATIO**

SAMPLE ID RD-11 SAMPLE DEPTH 2.0 - 5.0 FT

SOURCE MATERIAL Auger Cutting

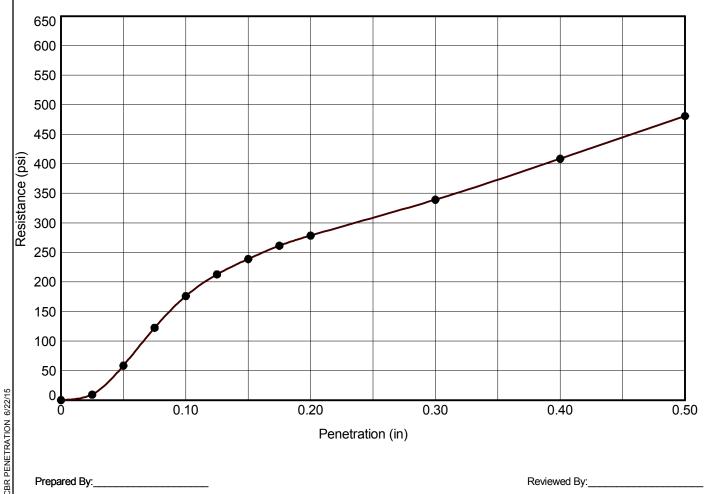
SAMPLE DESCRIPTION Clayey Sand With Gravel (SC)

Condition	Saturated	CBR Method	VTM 8
Surcharge Wt., (lb)	25	Proctor Method	VTM1
Initial Dry Density, pcf	134.1	Maximum Dry Density, pcf	135.7
Initial Moisture, %	8.0	Optimum Water Content, %	8.9
Initial Compaction, %	98.8	Percent Oversize, % No. 4 Sieve	22.1
CBR at 0.1 inch Penetration, %	21.5	Gravel, %	22.1
CBR at 0.2 inch Penetration, %	19.3	Fines, %	41.0
Swell, %	1.2	Liquid Limit	31
Top 1" Moisture After Penetration, %	13.8	Plastic Limit	21
Moisture After Penetration, %	10.4	Plasticity Index	10

Remarks:

Prepared By:\_

### Penetration (in) Vs. Resistance



Reviewed By:\_



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

### **CALIFORNIA BEARING RATIO**

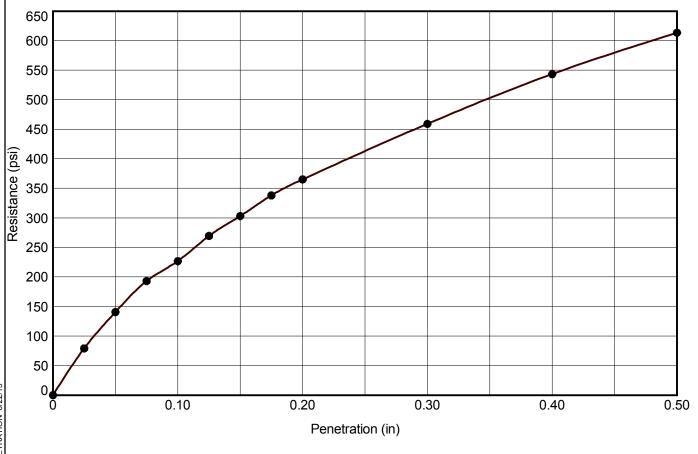
SAMPLE ID RD-22
SAMPLE DEPTH 1.0 - 4.0 FT
SOURCE MATERIAL Auger Cutting

SAMPLE DESCRIPTION Clayey Sand With Gravel (SC)

Condition	Saturated	CBR Method	VTM 8
Surcharge Wt., (lb)	25	Proctor Method	VTM1
Initial Dry Density, pcf	129.5	Maximum Dry Density, pcf	130.9
Initial Moisture, %	8.6	Optimum Water Content, %	9.4
Initial Compaction, %	98.9	Percent Oversize, % No. 4 Sieve	21.6
CBR at 0.1 inch Penetration, %	22.7	Gravel, %	21.6
CBR at 0.2 inch Penetration, %	24.3	Fines, %	43.3
Swell, %	0.8	Liquid Limit	33
Top 1" Moisture After Penetration, %	17.7	Plastic Limit	18
Moisture After Penetration, %	10.9	Plasticity Index	15

Remarks:

### Penetration (in) Vs. Resistance



**CBR PENETRATION 6/22/15** 

Prepared By:

Reviewed By:\_\_



PROJECT NO.: 01.02095.01 LOCATION: Leesburg, Virginia CLIENT: Rinker Design Associates, P. C.

### **CALIFORNIA BEARING RATIO**

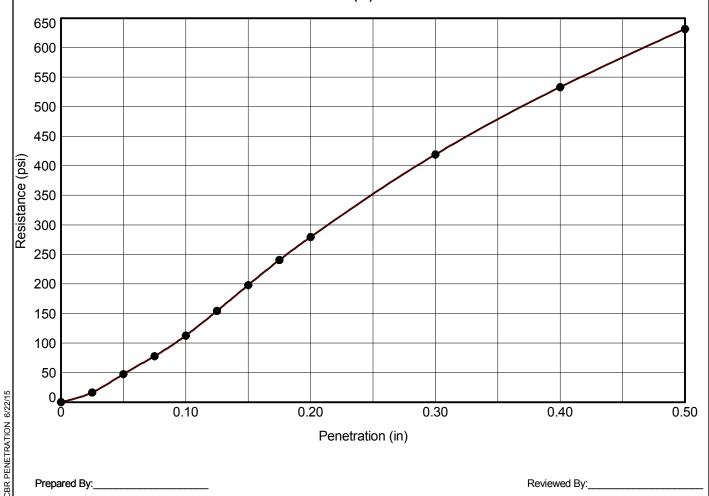
SAMPLE ID **RD-27** SAMPLE DEPTH 2.0 - 5.0 FT SOURCE MATERIAL Auger Cutting SAMPLE DESCRIPTION Clayey Sand With Gravel (SC)

Condition	Saturated	CBR Method	VTM 8
Surcharge Wt., (lb)	25	Proctor Method	VTM1
Initial Dry Density, pcf	129.4	Maximum Dry Density, pcf	127.7
Initial Moisture, %	9.3	Optimum Water Content, %	10.4
Initial Compaction, %	101.3	Percent Oversize, % No. 4 Sieve	24.0
CBR at 0.1 inch Penetration, %	17.1	Gravel, %	24.0
CBR at 0.2 inch Penetration, %	20.5	Fines, %	44.1
Swell, %	0.3	Liquid Limit	28
Top 1" Moisture After Penetration, %	17.7	Plastic Limit	18
Moisture After Penetration, %	10.9	Plasticity Index	10

Remarks:

Prepared By:\_

### Penetration (in) Vs. Resistance



Reviewed By:\_

# APPENDIX D PAVEMENT DESIGN CALCULATIONS

### Traffic Load Worksheet

Project Name: Sycolin Road Widening Phase IV

Project Location: Leesburg, Virginia DMY Project Number: 01.02095.01

Pavement Location: Sycolin Road Full Depth Pavement Design

INPUT DATA
Design Life (years) = 20
Initial Design Year = 2019
Initial Design Year AADT = 16236
Directional Ratio = 50.0%
Growth Rate (%) = 2
% Cars/Passernger Vehicles = 88.7
% Single Unit Trucks = 10.5
% Tractor Trailer Trucks = 0.8
ESAL Factor - Cars = 0.0002
ESAL Factor - Single Unit Trucks = 0.46
ESAL Factor - Tractor Trailer Trucks = 1.05
Travel Lanes (each direction) = 2
Lane Distribution Factor = 0.9

Total ESALs for Design Life (W18) = 3685390  ESALs - Cars/Passenger Vehicles = 11495  ESALs - Single Unit Trucks = 3129614  ESALs - Tractor Trailer Trucks = 544281  Remarks:	RESULTS			
ESALs - Single Unit Trucks = 3129614  ESALs - Tractor Trailer Trucks = 544281	Total ESALs for Design Life (W18) =	3685390		
ESALs - Tractor Trailer Trucks = 544281	ESALs - Cars/Passenger Vehicles =	11495		
	ESALs - Single Unit Trucks =	3129614		
Remarks:	ESALs - Tractor Trailer Trucks =	544281		
Remarks:				
Remarks:				
Remarks:				
	Remarks:			
Prepared by: PL	Prepared by:	PL		
Reviewed by: PZ	Reviewed by:	PZ		

Ye	ar	AADT	Total ADT
2019	1	16236	5926140
2020	2	16561	11970803
2021	3	16892	18136359
2022	4	17230	24425226
2023	5	17574	30839871
2024	6	17926	37382808
2025	7	18284	44056604
2026	8	18650	50863876
2027	9	19023	57807294
2028	10	19404	64889580
2029	11	19792	72113511
2030	12	20187	79481921
2031	13	20591	86997700
2032	14	21003	94663794
2033	15	21423	102483210
2034	16	21852	110459014
2035	17	22289	118594334
2036	18	22734	126892361
2037	19	23189	135356348
2038	20	23653	143989615
2039	21	24126	152795547
2040	22	24608	161777598
2041	23	25101	170939290
2042	24	25603	180284216
2043	25	26115	189816040
2044	26	26637	199538501
2045	27	27170	209455411
2046	28	27713	219570659
2047	29	28267	229888213
2048	30	28833	240412117

### Pavement Design Worksheet

Project Name: Sycolin Road Widening Phase IV

Project Location: Leesburg, Virginia DMY Project Number: 01.02095.01

Pavement Location: Sycolin Road Full Depth Pavement Design

Design Life (years) = 20
Total ESALs - W <sub>18</sub> = 3685390
Reliability - R (%) = 90
Standard Normal Deviate - Z <sub>R</sub> = -1.282
Standard Deviation - $S_0 = 0.49$
Initial Serviceability = 4.2
Terminal Serviceability = 2.8
Serviceability Loss - ΔPSI = 1.4
Design CBR = 6.7
Design M <sub>r</sub> = 10050
Log W <sub>18</sub> = 6.57
Right Side of AASHTO Equation = 6.57
Required SN = 4.11

Pavement Course	a1	Thickness	SN
SM 9.5	0.44	2	0.88
IM 19	0.44	2	0.88
BM 25	0.44	6	2.64
Existing AC	0.396		0.00
Existing HCC	0.5		0.00
OGDL-Bounded	0.1		0.00
GAB	0.12	12	1.44
СТАВ	0.2		0.00
			0.00
			0.00
Total		22	5.84

ОК

Prepared by:	PL
Reviewed by:	PZ

Remarks: Based on traffic data provided by RDA

### Pavement Design Worksheet

Project Name: Sycolin Road Widening Phase IV

Project Location: Leesburg, Virginia DMY Project Number: 01.02095.01

Pavement Location: Sycolin Road Resurface Pavement Design

Design Life (years) = 20
Total ESALs - W <sub>18</sub> = 3685390
Reliability - R (%) = 90
Standard Normal Deviate - Z <sub>R</sub> = -1.282
Standard Deviation - $S_0 = 0.49$
Initial Serviceability = 4.2
Terminal Serviceability = 2.8
Serviceability Loss - ΔPSI = 1.4
Design CBR = 6.7
Design M <sub>r</sub> = 10050
Log W <sub>18</sub> = 6.57
Right Side of AASHTO Equation = 6.57
Required SN = 4.11

Pavement Course	a1	Thickness	SN
SM 9.5	0.44	2	0.88
IM 19	0.44		0.00
BM 25	0.44		0.00
Existing AC	0.396	7.7	3.05
Existing HCC	0.5		0.00
OGDL-Bounded	0.1		0.00
GAB	0.12	12	1.44
CTAB	0.2		0.00
			0.00
			0.00
Total		21.7	5.37

ОК

Prepared by:	PL
Reviewed by:	PZ

Remarks: Based on traffic data provided by RDA

### Pavement Design Worksheet

Project Name: Sycolin Road Widening Phase IV

Project Location: Leesburg, Virginia DMY Project Number: 01.02095.01

Pavement Location: Sycolin Road Temporary Pavement Design

Design Life (years) = 1		
Total ESALs - W <sub>18</sub> = 168532		
Reliability - R (%) = <b>85</b>		
Standard Normal Deviate - Z <sub>R</sub> = -1.037		
Standard Deviation - $S_0 = 0.49$		
Initial Serviceability = 4.2		
Terminal Serviceability = 2.8		
Serviceability Loss - ∆PSI = 1.4		
Design CBR = 6.7		
Design M <sub>r</sub> = 10050		
Log W <sub>18</sub> = 5.23		
Right Side of AASHTO Equation = 5.23		
Required SN = 2.28		

Pavement Course	a1	Thickness	SN
SM 9.5	0.44	1.5	0.66
IM 19	0.44	0	0.00
BM 25	0.44	3	1.32
Existing AC	0.396		0.00
Existing HCC	0.5		0.00
OGDL-Bounded	0.1		0.00
GAB	0.12	6	0.72
CTAB	0.2		0.00
			0.00
			0.00
Total		10.5	2.70

ОК

Prepared by:	PL
Reviewed by:	PZ

Remarks: Based on traffic data provided by RDA